GENERAL NOTES

1 THE FOLLOWING NOTES SHALL APPLY TO ALL STRUCTURAL DRAWINGS.

3 THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS. ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE INTENDED TO AUGMENT, NOT SUPERSEDE, THOSE SHOWN ON THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS. DRAWINGS MAY NOT BE TO SCALE.

4 THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS AND NOTIFY THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.

5 IF MATERIAL QUANTITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE BETTER QUALITY AND/OR QUANTITY, STRENGTH OR NOTED SHALL BE PROVIDED.

6 THE STRUCTURAL MODIFICATIONS ARE DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER CONSTRUCTION IS FULLY COMPLETED. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING CONSTRUCTION, INCLUDING PROVISIONS FOR CHANGEABLE WEATHER UNTIL THE BUILDING IS CLOSED AND CONDITIONED .
THE CONTRACTOR SHOULD DESIGN, INSTALL AND SUBSEQUENTLY REMOVE ANY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS NECESSARY TO MAINTAIN SAFETY AND STRUCTURAL STABILITY DURING CONSTRUCTION.

7 IT SHALL BE THE SOLE RESPONSIBLY OF THE CONTRACTOR TO INITIATE, MAINTAIN AND SUPERVISE ALL SAFETY PROCEDURES, WHILE FOLLOWING ALL APPLICABLE SAFETY CODES, BUILDING CODES AND GOVERNING REGULATIONS WITH JURISDICTION OVER THE CONSTRUCTION SITE DURING ALL PHASES OF CONSTRUCTION. THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PROCEDURES.

8 ANY FRAMING SHOWN ON DRAWINGS THAT SUPPORT EQUIPMENT (WHETHER SUPPORTED ABOVE OR SUSPENDED BELOW), DESIGN LOADS, OPENINGS AND PENETRATIONS, AND STRUCTURAL MEMBERS IN ANY MANNER RELATED TO HVAC, PLUMBING, ELECTRICITY OR FIRE PROTECTION REQUIREMENTS IS BASED ON EQUIPMENT DESIGNED, SHOWN AND/OR SPECIFIED IN THE CONSTRUCTION DOCUMENTS.
ALL REQUIREMENTS FRAMING MAY NOT BE SHOWN. USING THE DETAILS PROVIDED ON THE STRUCTURAL DRAWINGS, THE GENERAL CONTRACTOR AND SUB-CONTRACTOR AND/OR EACH PRIME CONTRACTOR MUST COORDINATE AND INSTALL THE ACTUAL FRAMING REQUIRED FOR THE EQUIPMENT TO BE INSTALLED, AND INCLUDE COST FOR ALL REQUIRED FRAMING IN THE BID.

9 EXISTING BUILDING: INSTALL TEMPORARY SUPPORT AND OTHER MEASURES AS REQUIRED TO PREVENT DAMAGE TO THE EXISTING BUILDING DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION AND FINAL CLEARANCE OF REQUIRED NEEDLING, SHORING, UNDERPINNING OR BRACING OF THE EXISTING BUILDING. FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS AND CONDITIONS WHICH AFFECT THE NEW CONSTRUCTION PRIOR TO THE START OF WORK. EXISTING CONDITIONS SHOWN ON THE DRAWINGS ARE ASSUMED, HAVE NOT BEEN CONFIRMED, ARE NOT GUARANTEED AND MAY CONFLICT WITH THE NEW WORK REQUIRED. FIELD VERIFY THAT THE EXISTING FRAMING AFFECTED BY THE NEW WORK IS IN SOUND CONDITION AND DOES NOT DISPLAY VISIBLE SIGNS OF DISTRESS OR DETERIORATION OR HAS BEEN PREVIOUSLY MODIFIED OR ALTERED. IMMEDIATELY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN THE INFORMATION SHOWN ON THE DRAWINGS AND ACTUAL FIELD CONDITIONS; DO NOT PROCEED WITH THAT PORTION OF WORK UNTIL ALL DISCREPANCIES HAVE BEEN RESOLVED. THE CONTRACTOR SHOULD SUBMIT A FIELD SURVEY SHOWING ALL DISCREPANCIES BETWEEN THE EXISTING CONDITION AND THE NEW WORK. BASED ON THE REPORTED FIELD CONDITIONS, THE ARCHITECT WILL SUBMIT SUPPLEMENTAL INSTRUCTIONS FOR ALL WORK (NEW OR EXISTING) REQUIRING MODIFICATIONS.

10 THE STRUCTURAL DRAWINGS ARE ONE DISCIPLINE OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES CONTAIN ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND COORDINATE THE INFORMATION INDICATED IN THESE CONSTRUCTION DOCUMENTS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.

11 ALL DETAILS, SECTIONS AND NOTES INDICATED ON THE CONSTRUCTION DOCUMENTS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

12 SHOULD ANY OF DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES. THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

13 SPECIALTY ENGINEERED PRODUCTS
THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS WHICH SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
1. STEEL STAIRS
2. LADDERS
3. HANDRAILS
4. CLADDING SYSTEMS
5. MISCELLANEOUS STEEL

14 SPECIAL INSPECTIONS: NO SPECIAL INSPECTIONS ARE REQUIRED FOR STRUCTURAL ITEMS. REFER TO OTHER DISCIPLINES FOR SPECIAL INSPECTIONS OF NON-STRUCTURAL SYSTEMS WHERE REQUIRED.

15 SHOP DRAWINGS
A. SUBMIT THE FOLLOWINGS DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION:
1. CONCRETE MIX DESIGNS
2. CONCRETE REINFORCEMENT
3. WOOD TRUSSES
B. THE CONTRACTOR SHALL REVIEW AND ACCEPT FULL RESPONSIBILITY FOR DIMENSIONAL CORRECTNESS. ALL SHOP DRAWINGS MUST BEAR THE APPROVAL STAMP OF THE CONTRACTOR (TO INCLUDE INITIALS, DATE AND DISPOSITION), PRIOR TO REVIEW BY THE ARCHITECT OR ENGINEER. THE ENGINEER WILL RETURN ALL DRAWINGS, UN-REVIEWED, THAT DO NOT BEAR THE APPROVAL STAMP OF THE CONTRACTOR.

16 CHANGES AND MODIFICATIONS:
SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
SPLICING, CUTTING, NOTCHING AND OTHER ALTERATIONS TO STRUCTURAL MEMBERS IS PROHIBITED WITHOUT WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER.
ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.

17 CONTRACTOR SHALL BRACE ALL CONSTRUCTION AGAINST GRAVITY, LATERAL & UPLIFT LOADS UNTIL STRUCTURE IS COMPLETE.

DESIGN CRITERIA

THE STRUCTURAL DESIGN OF THIS BUILDING WAS BASED ON THE DESIGN CRITERIA:

A. BUILDING CODE
INTERNATIONAL BUILDING CODE 2018
B. DESIGN ROOF LOADS
DEAD LOADS 20 PSF
LIVE LOADS 20 PSF
C. DESIGN FLOOR LOADS
LIVE LOADS 100 PSF
D. ROOF SNOW LOAD DATA: N/A

E. WIND DESIGN DATA:
ULTIMATE DESIGN WIND SPEED, Vult 134 MPH
NOMINAL DESIGN WIND SPEED, Vasd 104 MPH
RISK CATEGORY: II
WIND EXPOSURE CATEGORY: C
INTERNAL PRESSURE COEFFICIENTS +-0.18
COMPONENTS AND CLADDING WIND PRESSURE:
SEE WIND LOAD TABLES ON THIS SHT.
WIND PRESSURES PER ASCE: 7-16
F. WIND-BORNE DEBRIS REGION (WITHIN HURRICANE-PRONE REGIONS)
1. WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE Vult IS 130 MPH OR GREATER; OR
2. IN AREAS WHERE Vult IS 140 MPH OR GREATER.
G. EARTHQUAKE DESIGN DATA:
SEISMIC IMPORTANCE FACTOR, Ie
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS,
Ss= 0.061g S1= 0.046g
SITE CLASS: D
Fa= 1.6 Fv= 2.4
SMs= 0.097 SM1= 0.11g
DESIGN SPECTRAL RESPONSE ACELERATION PARAMITERS,
SDs= 0.065g SD1= 0.074g
SEISMIC DESIGN CATEGORY: C
BASIC SEISMIC FORCE-RESISTING SYSTEM:
BASE SHEAR: N/A

CONCRETE

1 ALL CONCRETE SHALL BE IN ACCORDANCE WITH THE "AMERICAN CONCRETE INSTITUTE BUILDING CODE" (ACI 318) AND WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ASI 301) LATEST EDITIONS.

2 CONCRETE MEMBERS SHALL HAVE THE FOLLWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:
A. STRUCTURAL CONCRETE

CLASS	LOCATION	f'c (psi)
I	FOUNDATIONS	3000
II	INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED	4000
III	INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED	4000 (W/AIR)
IV	NOT USED	1500

B. ALL DEFORMED REINFORCING BARS: ASTM A615, GR. 60
C. ALL WELDED WIRE FABRIC: ASTM1064, DELIVERED IN FLAT SHEETS

ALL OTHER CONCRETE TO BE 4000 PSI UNLESS NOTED OTHERWISE

3 ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST ADOPTED EDITIONS OF THE STANDARDS AND MATERIAL SPEFICATIONS REFERENCED HEREIN.

4 CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 3. CONTRACTOR SHALL SUBMIT BACKUP DATA PER SECTION 26.4 OF ACI 318 LATEST EDITION.

5 ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GR. 60. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

6 CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

7 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST 12 INCHES UNLESS NOTED OTHERWISE.

8 REBAR LAP LENGTH SHALL BE 48 BAR DIAMETERS MINIMUM. UNLESS NOTED OTHERWISE ON DRAWINGS.

9 PROVIDE ACI STANDARD HOOKS UNLESS NOTED OTHERWISE ON DRAWINGS.

10 ALL CONCRETE WORK SHALL CONFORM TO ACI 318 LATEST EDITION "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE STRUCTURES", ACI 301 "SPECIFICATIONS FOR STRUCTRUAL CONCRETE FOR BUILDINGS," ACI 305 "HOT WEATHER CONCRETING," AND ACI 306 "COLD WEATHER CONCRETING."

11 ALL CONCRETE DETAILS SHALL CONFORM TO ACI 315 LATEST EDITION "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS NOTED OTHERWISE ON THE DRAWINGS.

12 CONTRACTOR SHALL REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND SIZES OF SLEEVES, OPENINGS, EMBEDDED ITEMS, SLAB RECESSES, SLOPES, ETC. THESE ITEMS SHAL BE COORDINATED WITH OTHER TRADES AND INSTALLED PRIOR TO CONCRETE PLACEMENT.

13 CONTRACTOR SHALL VERIFY ANCHOR BOLT SIZES AND LOCATIONS PRIOR TO CONCRETE PLACEMENT.

14 BAR LENGTHS PROVIDED ON DRAWINGS DO NOT INCLUDE HOOK LENGTH. HOOKS SHALL BE PROVIDED AT TOP BARS, AT BEAM ENDS, AND SLAB EDGES.

15 CONTRACTOR SHALL PROVIDE CHAIRS, BOLSTERS, SPACERS, ETC. AS REQUIRED TO SECURELY SUPPORT REINFORCEMENT. SUPPORT ITEMS ON EXPOSED CONCRETE SHALL BE PLASTIC SUPPORT REINFORCEMENT, TIPPED OR STAINLESS STEEL. IN HIGHLY CORROSIVE ENVIRONMENTS, SUPPORT ITEMS SHALL BE PLASTIC.

16 ONE COPY OF ALL CONCRETE TEST RESULTS SHALL BE SUBMITTED BY THE TESTING AGENCY DIRECTLY TO THE ENGINEER OF RECORD.

17 THE CONTRACTOR SHALL SEAL ALL HAIRLINE CRACKING IN CONCRETE SLAB ON GRADE WITH A CRACK SUPPRESSION KIT SUCH AS LATICRETE OR APPROVED EQUAL.

18 A COPY OF THE "FIELD REFERENCE MANUAL" ACI SP-15 LATEST EDITION SHALL BE KEPT BY THE CONTRACTOR ON SITE.

19 FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL.

20 CONCRETE COVER: UNLESS OTHERWISE NOTED, DETAIL REINFORCING TO PROVIDE MINIMUM COVER AS FOLLOWS:
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3 INCHES
B. CONCRETE EXPOSED TO EARTH OR WEATHER
#5 BARS AND SMALLER 1-1/2 INCHES
OTHERS 2 INCHES
C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS 1-1/2 INCHES
SLABS, WALLS AND JOISTS:
#11 BARS AND SMALLER 3/4 INCHES
OTHERS 1-1/2 INCHES

21 SLUMP, (AT POINT OF PLACEMENT): 5" (+/- 1")

22 CALCIUM CHLORIDE SHALL NOT BE USED IN ANY FORM.

23 EXPANSION BOLTING:
UNLESS NOTED OTHERWISE ON PLANS OR IN SECTIONS, ALL EXPANSION BOLTS IN CONCRETE SHALL BE "HILTI KWIK BOLT 3 EXPANSION ANCHORS". ANCHORS USED IN EXTERIOR APPLICATIONS SHALL BE HOT-DIP GALVANIZED CARBON STEEL OR STAINLESS STEEL.

24 UNLESS NOTED OTHERWISE ON PLANS OR IN SECTIONS, ALL EPOXY ANCHORS FOR USE IN APPLICATIONS INVOLVING CONCRETE SHALL USE EITHER:
A) HILTI HIT HY 200 ADHESIVE ANCHORING SYSTEM.
B) SIMPSON STRONG-TIE AT HIGH STRENGTH ACRYLIC-TIE ANCHORING ADHESIVE.
INSTALL ALL ANCHORS FOLLOWING MANUFACTURER'S INSTALLATION INSTRUCTIONS

SAWN LUMBER

1 ALL GRADES OF LUMBER SHALL BE RATED BY THE SOUTHERN PINE INSPECTION BUREAU (SPIB), OR THE WESTERN WOOD PRODUCTS ASSOCIATION (WWPA). IUMBER GRADES SHALL BE AS FOLLOWS, WITH A MAXIMUM MOISTURE CONTENT OF 19%:

A. SOUTHERN PINE NO. 1
B. DOUGLAS FIR-LARCH NO. 1
C. HEM-FIR NORTH NO. 1

2 BOLT HEADS AND NUTS BEARING ON WOOD SHALL BE PROVIDED WITH STANDARD CUT WASHERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.

3 MINIMUM NAILED CONNECTIONS FOR WOOD FRAMING MEMBERS SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE LATEST EDITION.

FOUNDATION PLAN NOTES

1. DESIGN SOIL BEARING PRESSURE IS 2,000 PSF PER THE GEOTECHNICAL REPORT.

2. A GEOTECHNICAL REPORT ON THE SUBSURFACE SOILS CONDITIONS HAS BEEN PREPARED FOR THIS PROJECT BY NOVA ENGINEERING AND ENVIRONMENTAL REPORT NO.10111-2023102 DATED: AUGUST 24, 2023

3. ALL CONTRACTORS INVOLVED WITH THE EXCAVATION, BACKFILL AND FOUNDATION CONSTRUCTION SHALL FAMILIARIZE THEMSELVES WITH THIS REPORT.

4. ALL WORK REGARDING SITE PREPARATION, EARTH FILL CONSTRUCTION, BACKFILL REQUIREMENTS, FOUNDATION PREPARATIONS, ETC., SHALL BE IN STRICT CONFORMANCE TO THE GEOTECHNICAL ENGINEER'S REPORT. THE REPORT IS PART OF THE CONTRACT DOCUMENTS.

5. FOUNDATION BEARING SURFACES SHALL BE INSPECTED UNDER THE DIRECTION OF THE PROJECT GEOTECHNICAL ENGINEER.

6. IF NOT OTHERWISE SPECIFIED IN GEOTECHNICAL REPORT, COMPACT ALL FILL TO 98% OF OPTIMUM LABORATORY DENSITY IN ACCORDANCE WITH ASTM D698 STANDARD PROCTOR METHOD.

7. DO NOT BACKFILL AGAINST FOUNDATION OR BASEMENT WALLS UNTIL FLOOR SLAB/FRAMING IS IN PLACE OR PERMISSION IS OBTAINED FROM THE ARCHITECT.

8. EXCEPT FOR CANTILEVER RETAINING WALLS OR UNLESS NOTED OTHERWISE ON DRAWINGS, SUBGRADE WALLS ARE SUPPORTED BY THE FLOOR ABOVE AND BELOW. PROPER TEMPORARY BRACING MAY BE USED IN LEIU OF THE FLOOR SUPPORT BASED ON THE DESIGN BY A PROFESSIONAL ENGINEER.

9. ALL FOOTINGS ARE TO REST ON FIRM, LEVEL, UNDISTURBED SOIL OR ENGINEERED FILL, OF BEARING CAPACITY NOTED, REGARDLESS OF ELEVATIONS SHOWN ON DRAWINGS.

10. WHERE ROCK OR SHALE IS THE BEARING MATERIAL, PROVIDE 6" DEEP PENETRATION.

11. WHERE FOOTINGS ARE IN CLOSE PROXIMITY OF SEWERS, BOTTOM OF FOOTINGS SHALL BE AT LEAST 8" BELOW INVERT ELEVATION OF SEWERS.

12. EXCAVATIONS FREE OF WATER AT ALL TIMES.

13. USE LEAN CONCRETE (F'C'=1500 PSI) FOR OVER-EXCAVATION OF FOOTINGS AND REPLACEMENT OF WEAKENED...

14. PROVIDE POSITIVE DRAINAGE AWAY FROM EXCAVATIONS SO AS NOT TO ALLOW STANDING WATER FOR LONG PERIODS OF TIME.

15. PROVIDE A 10 MIL THICK VAPOR BARRIER BETWEEN THE COMPACTED BASE AND CONCRETE SLAB.

16. DO NOT PUNCTURE THE VAPOR BARRIER. LAP AND TAPE ENDS.

17. PERFORM ALL SITEWORK UNDER THE DIRECT SUPERVISION OF A GEOTECHNICAL ENGINEER.

PRE-ENGINEERED TRUSSES

ROOF AND/OR FLOOR TRUSSES FABRICATED TO ACHIEVE THE ROOF AND/OR FLOOR PLANES DEPICTED ON THE ARCHITECTURAL PLANS SHALL BE DESIGNED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER. FABRICATION DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH ANSI/TPI1-1995 AND SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. THE TRUSS SYSTEM ENGINEER SHALL CERTIFY THE PRE-ENGINEERED TRUSS SYSTEM AND INDIVIDUAL TRUSSES TO MEET OR EXCEED THE FOLLOWING CRITERIA:

1. ALL TRUSS FRAMING MEMBERS SHALL BE A MINIMUM, NO. 2 GRADE NON TREATED CONVENTIONAL LUMBER SOUTHERN YELLOW PINE, 19 % M.C.

2. STAINLESS STEEL METAL HANGERS AND FRAMING ANCHORS SHALL BE USED AND SHALL BE FASTENED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS.

3. ANCHORING AND NAILING NOT SPECIFIED SHALL COMPLY WITH THE NAILING SCHEDULE GIVEN WITHIN FLORIDA BUILDING CODE.

4. TRUSS SYSTEM LAYOUT AND INDIVIDUAL TRUSS DRAWINGS SHALL BE SUBMITTED BY A LICENSED ENGINEER FOR REVIEW PRIOR TO FABRICATION.

5. THE TRUSS DESIGNS SHALL SHOW AS A MINIMUM THE FOLLOWING:
A. THOSE ITEMS, AS LISTED, IN ANSI/TPI-1-2214 UNDER SECTION 2.1 (TRUSS DESIGNER RESPONSIBILITIES) WHICH INCLUDED SECTION 2.1.2.12: THE LOCATION FOR CONTINUOUS PERMANENT LATERAL BRACING OF TRUSS MEMBERS SUBJECT TO BUCKLING DUE TO COMPRESSIVE FORCES.
B. THE NAME, ADDRESS, AND LICENSE NUMBER OF THE TRUSS SYSTEM ENGINEER OF RECORD, AND THE NAME: ADDRESS, AND LICENSE OF THE TRUSS DESIGN ENGINEER.
C. COMPLETE ADDRESS OF THE PROJECT.
D. IDENTIFICATION OF THE APPLICABLE CODE AND CHAPTER(S) WHICH THE THE TRUSS DESIGN IS INTENDED TO MEET. THE DESIGN CRITERIA RELIED UPON IN DESIGNING THE TRUSSES, AND THE TRUSS DESIGN LOADING
E. IDENTIFICATION OF ANY COMPUTER PROGRAMS USED FOR ENGINEERING THE TRUSS DESIGNS.
F. AN INDEX OF THE ATTACHED TRUSS DESIGN DRAWINGS, THE NAMING AND NUMBERING SYSTEM UTILIZED FOR THE DRAWINGS SHALL BE CLEAR AS TO THE HOW MANY DRAWINGS THERE ARE IN THE SET AND THE DATE AND SEQUENCE NUMBER OF EACH OF THESE DRAWINGS SHALL BE INCLUDED.

6. DESIGN GRAVITY LOADING:
TOP CHORD: DEAD LOAD = 10 PSF
LIVE LOAD = 20 PSF
BOTTOM... DEAD LOAD = 10 PSF
LIVE LOAD = 5 PSF (NON-CONCURRENT)
NON-CONCURRENT LIVE LOAD = 10 PSF

SEE ARCH. DWGS FOR ADDITION LOADS DUE TO FIRE RATED CONVERINGS APPLIED TO PRE-ENGINEERED TRUSSES

7. THE TRUSS MANUFACTURER SHALL MANUFACTURER EACH TRUSS IN ACCORDANCE WITH CHAPTER 3 OF ANSI/TPI-1-2014 AND PROVIDE ALL THE REQUIREMENTS OF SECTION 2.4.3 OF ANSI/TPI-1-2014 (PLACEMENT PLANS SHALL BE PROVIDED)

8. TEMPORARY BRACING AND OTHER CONSTRUCTION MEANS AND METHODS ARE OUTSIDE THE SCOPE OF THIS DOCUMENT. SHOULD THE CONTRACTOR DESIRE ADVICE CONCERNING INSTALLATION OF THE TRUSSES, CONTACT THE ENGINEER OF RECORD.

STRUCTURAL WOOD

1. WOOD FRAMING SHALL COMPLY WITH THE SOUTHERN PINE INSPECTION BUREAU, OR SHALL CONFORM TO SPECIFICATIONS AS PUBLISHED BY THE WESTERN WOODS PRODUCTS ASSOCIATION.

2. WOOD FRAMING 2 INCHES X 4 INCHES AND LARGER SHALL BE NO. 2 SOUTHERN PINE, NO. 2 DOUGLAS FIR LARCH, OR EQUIVALENT (U.N.O.).

3. WOOD COLUMNS 6 INCHES X 6 INCHES AND LARGER SHALL BE NO. 1 SOUTHERN PINE, NO. 1 DOUGLAS FIR LARCH, OR EQUIVALENT.

4. ALL EXPOSED WOOD RAFTERS AND COLUMNS SHALL BE "SELECT" GRADE AS DESCRIBED IN AITC.

5. ALL PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED (USE CATEGORY 2 AS SPECIFIED BY AWPA) FOR MOISTURE PROTECTION. ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (USE CATEGORY 3B AS SPECIFIED BY AWPA) OR WESTERN RED CEDAR.

6. GLUED-LAMINATED MEMBERS SHALL BE INDUSTRIAL APPEARANCE, E1.8, WITH GRADES AND SPECIES AS FOLLOWS, U.N.O. MEMBER SIZES BASED ON CURRENT NDS WESTERN SPECIES

· SIMPLY SUPPORTED SPAN MEMBERS:

SPECIES	GRADE
DOUGLAS FIR	24F-V4
SOUTHERN PINE	24F-V3

· MULTI SPAN MEMBERS:

SPECIES	GRADE
DOUGLAS FIR	24F-V8
SOUTHERN PINE	24F-V8

7. PRE-FABRICATED METAL PLATED WOOD TRUSSES
A. TRUSS FABRICATION AND INSTALLATION SHALL COMPLY WITH THE FOLLOWING STANDARDS:
1. ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION"
2. TPI HIB "COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES".
3. TPI DSB "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD...
B. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
1. TOP CHORD: LIVE LOAD = 20 PSF
DEAD LOAD = 10 PSF
2. BOTTOM CHORD: LIVE LOAD = 5 PSF (NON-CONCURENT)
DEAD LOAD = 10 PSF
3. REFERENCE ROOF PLAN FOR ADDITIONAL MECHANICAL EQUIPMENT LOADS
4. REQUIRED UPLIFT
C. ALL TRUSS-TO-TRUSS CONNECTORS SHALL BE SPECIFIED BY THE TRUSS MANUFACTURER. TRUSS-TO-FRAMING CONNECTORS SHALL BE SPECIFIED BY THE ENGINEER OF RECORD, UNLESS SPECIFICALLY NOTED.
D. BOTTOM CHORD TENSION SPLICES ARE PROHIBITTED WITHIN THE MIDDLE 13 OF THE SPAN.
E. FABRICATOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF MEMBERS, BRIDGING, BRACING, ERECTION DETAILS, TRUSS PENETRATIONS, AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER (STATE).
8. ROOF DECK:
A. ALL ROOF DECK SHALL BE APA RATED STRUCTURAL I GRADE PLYWOOD OR OSB (ORIENTED STRAND BOARD). STRUCTURAL I GRADES MAY HAVE EITHER AN EXTERIOR OR EXPOSURE I DESIGNATION (U.N.O.).
B. ROOF SHEATHING SHALL BE 23 32 INCH THICK MINIMUM (48/24) U.N.O.
C. STAGGER ENDS OF SHEETS
D. PROVIDE BLOCKING AT EDGES OF ALL ROOF SHEETS. PLYWOOD CLIPS MAY BE USED AT ROOF INSTEAD OF BLOCKING, UNLESS BLOCKING REQUIRED FOR NAILING.
E. NAIL EDGES OF ROOF SHEETS AT 6 IN. O.C. MAXIMUM (U.N.O.).
F. NAIL FACES OF ROOF SHEETS AT 12 IN. O.C. MAXIMUM.
G. USE MINIMUM 10D COMMON NAILS (U.N.O.).
9. WALL SHEATHING
A. ALL WALL SHEATHING SHALL BE APA RATED STRUCTURAL I GRADE PLYWOOD OR OSB (ORIENTED STRAND BOARD)....
B. WALL SHEATHING SHALL BE 15 32 INCH THICK MINIMUM (32/16) U.N.O.
C. STAGGER ENDS OF SHEETS.
D. PROVIDE BLOCKING AT EDGES OF ALL SHEARWALL SHEETS.
E. NAIL EDGES OF SHEARWALL SHEETS PER SCHEDULE ON PLAN (OTHER WALLS AT 6 IN. O.C. MAXIMUM).
F. NAIL FACES OF WALL SHEETS AT 12 IN. O.C. MAXIMUM.
G. USE MINIMUM 8D COMMON NAILS (U.N.O.).
10. CONNECTORS SHALL BE AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL. CONNECTORS USED WITH PRESSURE TREATED LUMBER OR IN UNCONDITIONED SPACE, SHALL HAVE THE ZMAX (6185) COATING.
11. NAILING, UNLESS NOTED OTHERWISE, SHALL BE PER THE 2012 INTERNATIONAL BUILDING CODE.
12. ANCHOR BOLTS SHALL BE ASTM F1554, TYPICAL U.N.O.

Jose Miguel Ortiz State of Florida, Professional Engineer, License No. 67920. This item has been electronically signed and sealed by Jose Miguel Ortiz on the date indicated here using a SHA authentication code. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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Department of Environmental Protection
Division of Recreation and Parks
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DRAWN BY:	C.P.H.	COMP FILE No: F13633
REVIEWED BY:	C.P.H.	START PROJECT No: 61437C

PROFESSIONAL ENGINEER CERTIFICATION
JOSE MIGUEL ORTIZ
LICENSE No. 67920
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jose M Ortiz
Digitally signed by Jose M Ortiz
DN: c=US, o=cph, ou=Jose M Ortiz, cn=Jose M Ortiz
Reason: I am the author of this document
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PROJECT TITLE: CONSTITUTION CONVENTION STATE PARK
SHEET TITLE: GENERAL NOTES
PROJECT No: CONSTITUTION CONVENTION STATE PARK PORT ST. JOE / GULF COUNTY / FLORIDA

SHEET NO.
S001

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION

LIST OF STRUCTURAL ABBREVIATIONS

(ABBREVIATIONS SHOWN ARE NOT NECESSARILY USED ON THE DRAWINGS)

#	NUMBER
&	AND
@	AT
A.B.	ANCHOR BOLTS
A.C.I.	AMERINCAN CONCRETE INSTITUTE
A.I.S.C.	AMERICAN INSTITUTE OF STL CONSTRUCTION
A.I.S.I.	AMERICAN IRON AND STEEL INSTITUTE
A.W.S.	AMERICAN WELDING SOCIETY
ALT.	ALTERNATIVE
ALUM.	ALUMINUM
ARCH.	ARCHITECTURAL
ANCH.	ANCHOR
APPROX.	APPROXIMATELY
B1 (1, 2,...)	BEAM SCHEDULE MARK
BETW/	BETWEEN
BLDG.	BUILDING
BLK.	BLOCK
BM.	BEAM
BOT.	BOTTOM
BRG.	BEARING
BS	BOTH SIDES
BTJ	BOLTED TIE JOIST
BRK	BRICK
BRDG	BRIDGING
BP	BASE PLATE
C. SC	STEEL COLUMN
CF	CUBIC FEET
C.I.P.	CAST IN PLACE
C.L.	CENTERLINE
CMU	CONCRETE MASONRY UNIT
CY	CUBIC YARDS
C1 (1, 2,...)	STEEL COLUMN SCHEDULE MARK
CB1 (1, 2,...)	CONCRETE BEAM SCHEDULE MARK
CC1 (1, 2,...)	CONCRETE COLUMN SCHEDULE MARK
CJ	CONSTRUCTION OR CONTROL JOINT
CLR	CLEAR
COL	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
CONTR.	CONTRACTOR
CEIL	CEILING
CANT.	CANTILEVER
CHAN.	CHANNEL
COMPR.	COMPRESSION
CORRD.	COORDINATE
CTR	CENTER
CTRD	CENTERED
DEPT.	DEPARTMENT
DIA	DIAMETER
DIM.	DIMENSION
DIST.	DISTANCE
DN.	DOWN
DR.	DRAIN
DTL	DETAIL
DWG	DRAWING
DT	DOUBLE TEE
DWL	DOWEL
E.F.	EACH FACE
E.W.	EACH WAY
EA	EACH
E.J.	EXPANSION JOINT
E.E.	EACH END
EL./ELEV.	ELEVATION
ELEC.	ELECTRICAL
ENGR.	ENGINEER
EOA	END OF ANGLE
EOJ	END OF JOIST
EQ	EQUAL
EQUIV	EQUIVALENT
EQ. SP.	EQUAL SPACING
EXP	EXPANSION
EXIST.	EXISTING
EXT.	EXTERIOR
EMBED	EMBEDMENT
EOP	EDGE OF PIT
E.S.	EACH SIDE
F.D.	FLOOR DRAIN
F1 (1, 2,...)	FOOTING SCHEDULE MARK
FIN. FLR.	FINISHED FLOOR
FIN. GR.	FINISHED GRADE
FLR.	FLOOR
FDN	FOUNDATION
FRMG	FRAMING
F.S.	FAR SIDE
FT.	FEET
FTG.	FOOTING
GA.	GAUGE
GALV.	GALVANIZED
GEN.	GENERAL
GR.	GRADE
GRTG.	GRATING
HOR/HORIZ.	HORIZONTAL
HT.	HEIGHT
H.S.	HEADED STUD
HGR.	HANGER
HI	HIGH
HK	HOOK
HPT	HORIZONTAL POINT
HR	HANDRAIL
HSA	HEADED STUD ANCHOR
I.D.	INSIDE DIAMETER
IN.	INCH(ES)
INT.	INTERIOR
I.T.	INVERTED TEE
JST.	JOIST
JT	JOINT
K	KIP
KB	KNEE BRACE
L	STEEL ANGLE
LGTH	LENGTH
LT. GA.	LIGHT GAUGE
LB	LEDGER BEAM OR LINTEL BEAM
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LNTL	LINTEL
LO	LOW
LSL	LONG SLOTTED HOLES
LT WT	LIGHTWEIGHT
LVL	LAMINATED VENEER LUMBER (1.9E)
LW	LONG WAY
MAT.	MATERIAL
MAX	MAXIMUM
MB1 (1, 2,...)	MASONRY BEAM SCHEDULE MARK
MC1 (1, 2,...)	MASONRY COLUMNSCHEDULE MARK
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MCJ	MASONRY CONTROL JOINT
M.O.	MASONRY OPENING
MEZZ	MEZZANINE
MS	METAL STUD
N.I.C.	NOT IN CONTRACT
NOM.	NOMINAL
NTS	NOT TO SCALE
NS	NEAR SIDE
NS/FS	NEAR SIDE AND FAR SIDE
NDS	NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
NO.	NUMBER
N.WT.	NORMAL WEIGHT
NSG	NON-SHRINK GROUT
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
OPG.	OPENING
OPP	OPPOSITE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER (2.0E)
P.T.	PRESSURE TREATED
P.C.	PRECAST (CONCRETE)
PAR.	PARALLEL
PL	PLATE
PLYWD	PLYWOOD
PREFAB	PREFABRICATED
PLUMB	PLUMBING
PAF	POWDER ACTUATED FASTENER
PROP.	PROPERTY
PROP. L.	PROPERTY LINE
R.D.	ROOF DRAIN
REF.	REFERENCE
REINF.	REINFORCEMENT
REQ'D.	REQUIRED
RF.	ROOF
R	RISER
RAD	RADIUS
RTN	RETURN
RW	RETAINING WALL
S.F.	SQUARE FEET
SS	STAINLESS STEEL
SECT.	SECTION
SCH.	SCHEDULE
SPC	STANDARD PIPE COLUMN
SPECS	SPECIFICATIONS
STD	STANDARD
STL.	STEEL
STRUCT.	STRUCTURAL
SYM	SYMMETRICAL
S.E.	SLAB EDGE
SHRW	SHEAR WALL
SHT.	SHEET
SIM.	SIMILAR
SJ	SAWCUT CONTROL JOINT
SJI	STEEL JOIST INSTITUTE
SL	SLOPE
SOG	SLAB ON GRADE
SPA	SPACE
SQ.	SQUARE
STR	STIRRUP
STR	STRENGTH
SW	SHORT WAY
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
T/	TOP OF
T/BM	TOP OF BEAM
T/CB	TOP OF CONCRETE BEAM
T/FTG.	TOP OF FOOTING
T/SLAB	TOP OF SLAB
T/STL	TOP OF STEEL
T/TB	TOP OF TIE BEAM
TB1 (1, 2,...)	CONC. TIE BEAM SCHEDULE MARK
TDS1 (1, 2,...)	TURNDOWN SLAB SCHEDULE MARK
THK	THICK
TS	TUBE STEEL
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
V.I.F.	VERIFY IN FIELD
W.P.	WORKING POINT
W/	WITH
W/O	WITHOUT
WD.	WOOD
WF1 (1, 2,...)	WALL FOOTING SCHEDULE MARK
WWF	WELDED WIRE FABRIC
XSPC	EXTRA STRONG PIPE COLUMN
XXSPC	DOUBLE EXTRA STRONG PIPE COLUMN
YD	YARD
YP	YIELD POINT

SPECIAL INSPECTIONS

1. SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 110 OF FBC 2020 AND THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED UNDER SECTION 1704. THE FOLLOWING AREAS OF WORK REQUIRE SPECIAL INSPECTIONS IN ACCORDANCE WITH THE LISTED 2018 IBC SECTIONS/LOCATIONS:
   - A. SOILS - SECTION 1705.6 PER TABLE 1705.6.
   - B. CONCRETE - SECTION 1705.3 PER TABLE 1705.3.
   - C. STEEL - SECTION 1705.2 PER TABLE 1705.2.3.
   - D. MASONRY - SECTION 1705.4.
   - E. WOOD - SECTION 1705.5.

CONTRACTOR NOTES FOR EXISTING CONDITIONS

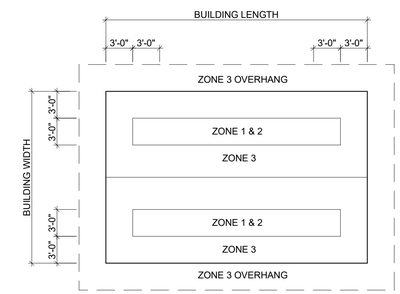
1. PRIOR TO CONSTRUCTION THE CONTRACTOR MUST VERIFY THAT THE EXISTING STRUCTURAL FRAMING CONDITIONS MATCH WHAT IS SHOWN IN THE STRUCTURAL DRAWINGS.
2. G.C. SHALL CONTACT ENGINEER OF RECORD IN THE EVENT THE SIZING IS NOT CLEAR.
3. THE G.C. SHALL CONTACT THE ENGINEER OF RECORD OF ANY CONDITION THEY ENCOUNTER IF THE MEMBERS INTEGRITY IS COMPROMISED.
4. THE G.C. SHALL CONTACT THE ENGINEER OF RECORD OF ANY CONDITION THEY ENCOUNTER IF IT DIFFERS FROM THESE DRAWINGS.

ASCE 7-16 / ULTIMATE COMPONENT AND CLADDING ROOF PRESSURES

ZONE	EFFECTIVE WIND AREA	+CN	-CN	ULTIMATE WP (PSF)		NOMINAL WP (PSF)	
1	<=9 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9
1	18 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9
1	>36 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9
2	<=9 sf	1.77	-1.63	49.9	-46.1	29.9	-27.7
2	18 sf	1.77	-1.63	49.9	-46.1	29.9	-27.7
2	>36 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9
3	<=9 sf	2.29	-2.11	64.7	-59.5	38.8	-35.7
3	18 sf	1.77	-1.63	49.9	-46.1	29.9	-27.7
3	>36 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9

ASCE 7-16 / ULTIMATE COMPONENT AND CLADDING ROOF OVERHANG PRESSURES

ZONE	EFFECTIVE WIND AREA	+CN	-CN	ULTIMATE WP (PSF)		NOMINAL WP (PSF)	
3 overhang	<=9 sf	2.29	-2.11	64.7	-59.5	38.8	-35.7
3 overhang	18 sf	1.77	-1.63	49.9	-46.1	29.9	-27.7
3 overhang	>36 sf	1.15	-1.05	32.3	-29.8	19.4	-17.9



PLAN VIEW - ROOF

Department of Environmental Protection
Division of Recreation and Parks
Bureau of Design and Construction
3800 Commonwealth Blvd., Tallahassee, FL 32399 (851) 425-2300

SYMBOL	DATE	REVISION
A		
B		
C		
D		

DESIGNER: Designer
DRAWN BY: Author
REVIEWED BY: Checker
ISSUE DATE: 07/26/2023
COMP FILE No: F13633
START PROJECT No: 61437C

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PROFESSIONAL ENGINEER

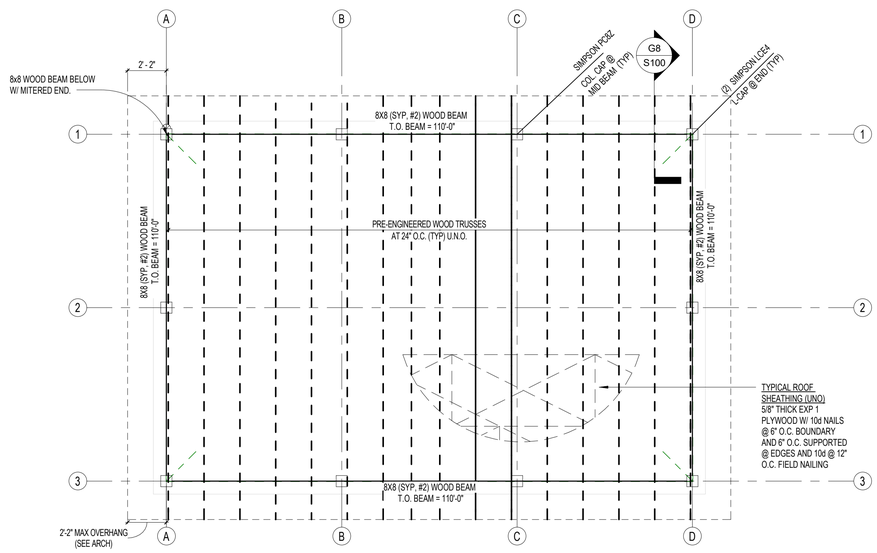
CONSTITUTION CONVENTION STATE PARK

SHEET TITLE: GENERAL NOTES

PROJECT TITLE: CONSTITUTION CONVENTION STATE PARK
PORT ST. JOE / GULF COUNTY / FLORIDA

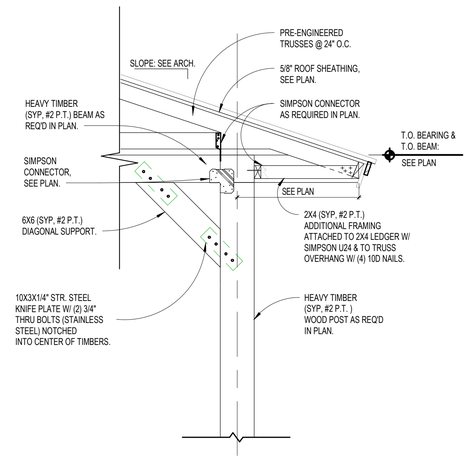
SHEET NO.
S002

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION



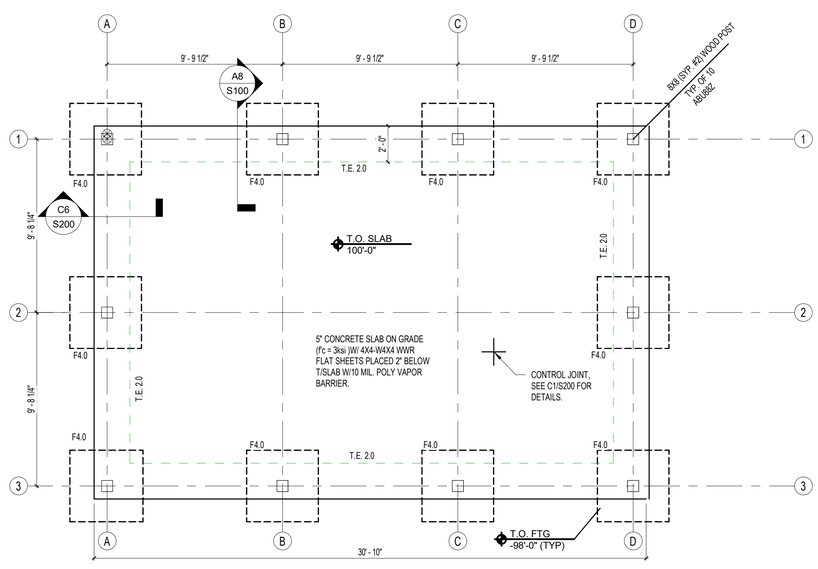
G1 FRAMING PLAN

1/4" = 1'-0"



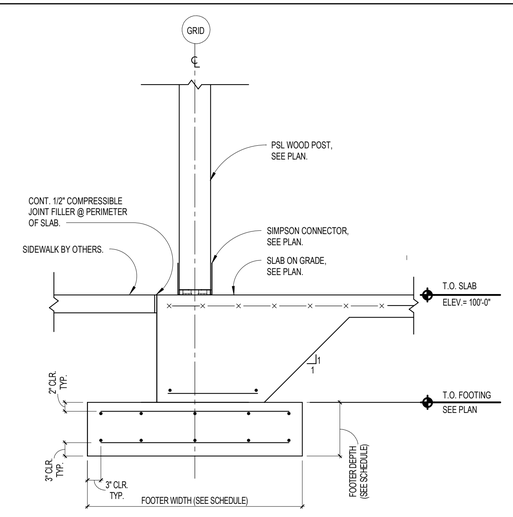
G8 SECTION AT POST

3/4" = 1'-0"



A1 FOUNDATION PLAN

1/4" = 1'-0"



A8 FOOTER SECTION

3/4" = 1'-0"

FOOTING SCHEDULE

TYPE MARK	W (WIDTH)	L (LENGTH)	T (FOOTING THICKNESS)	REINF. (LONGITUDINAL BARS)	RT (TRANSVERSE BARS)
TE2.0	2'-0"	CONT.	24"	(3)#5 @ BOT.	#5 @ 12" O.C.
F4.0	4'-0"	4'-0"	18"	(5)#5 @ TOP & BOT.	#5 @ 12" O.C.@TOP & BOT.

WOOD POST SCHEDULE

MARK	DIMENSIONS	MATERIAL	REMARK
WP	8" X 8"	2.0E, PSL	ABU882

LEGEND

TEX.0	DENOTES THICKENED EDGE SLAB, REFER TO SCHEDULE FOR DIMENSION AND REINFORCEMENT.
F#	DENOTES ISOLATED FOOTER, REFER TO SCHEDULE FOR DIMENSION AND REINFORCEMENT.

WOOD FRAMING PLAN NOTES

1. THE GENERAL CONTRACTOR SHALL VERIFY ALL RTU WEIGHTS, LOCATIONS, CURB DIMENSIONS, SUPPORT RACK DIMENSIONS, ROOF OPENING SIZES AND ROOF OPENING LOCATIONS WITH MECH, ARCH, AND SUPPLIER PRIOR TO FABRICATION AND CONSTRUCTION. REPORT ANY DISCREPANCIES TO ARCHITECT AND ENGINEER.
2. THE TRUSS LAYOUT ABOVE IS USED ONLY FOR THE TYPE AND LOCATION OF CONNECTORS REQUIRED. SEE SIGNED AND SEALED TRUSS LAYOUT PROVIDED BY TRUSS SYSTEM ENGINEER FOR CONSTRUCTION.
3. TRUSS SYSTEM ENGINEER SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS FOR REVIEW BY ARCHITECT / ENGINEER PRIOR TO CONSTRUCTION.
4. ALL PLYWOOD ROOF SHEATHING SHALL BE PRESSURE TREATED 5/8 INCH "APA RATED SHEATHING", 32/16 SPACING. ALL PANELS SHALL BE NAILED WITH 10D (STAINLESS STEEL) NAILS AT 4 INCHES O.C. AT ALL ENDS AND EDGES AND AT 6 INCHES O.C. AT ALL INTERMEDIATE SUPPORTS. ALL PLYWOOD PANEL END JOINTS SHALL OCCUR OVER SUPPORTS AND SHALL BE STAGGERED ONE HALF PANEL LENGTH FROM ADJACENT PANELS.
5. ALL TRUSS-TO-TRUSS CONNECTIONS ARE PROVIDED BY TRUSS SYSTEM ENGINEER.
6. ALL BUILDING ELEVATIONS AND DOOR/WINDOW LOCATIONS & SIZES ARE ILLUSTRATED WITHIN THE STRUCTURAL DRAWINGS FOR REFERENCE ONLY, REFER TO ARCHITECTURAL DRAWINGS FOR THE ACTUAL.
7. CONTRACTOR SHALL COORDINATE WITH TRUSS DESIGNER (MANUFACTURER) TO AVOID POTENTIAL CONFLICT OF MECHANICAL DUCTS CROSSING BETWEEN WOOD TRUSSES.
8. ALL ROOF TRUSSES, GIRDERS, AND/OR RAFTERS SHALL BE ANCHORED TO TOP OF STRUCTURAL BEARING WALL/BEAM AS FOLLOWS:
   COMMON PRE-ENG. TRUSSES TO WOD BEAM: (1) H14 STRAP
9. VERIFY ALL TOP OF PARAPET AND WALL BEARING ELEVATION W/ ARCH. DWGS AND REPORT DISCREPANCIES PRIOR TO FABRICATION AND CONCTRUCTION.

FOUNDATION PLAN NOTES

1. TOP OF SLAB = 100'-0". 100'-0" IS FOR REFERENCE ONLY. SEE CIVIL FOR NVGD ELEVATION.
2. TOP OF FOOTINGS @ EL. (-2'-0") MIN. BELOW TOP OF SLAB UNLESS NOTED OTHERWISE. BELOW ADJACENT FINISH GRADE. VERIFY ALL TOP OF FOOTING ELEVATIONS W/ ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION AND CONSTRUCTION.
3. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS W/ ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
4. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL SLEEVES, PIPES, INSERTS AND EMBEDDED ITEMS.
5. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SLOPES AND RECESSES. SEE PLUMBING DRAWINGS FOR EXACT LOCATIONS.
6. SEE SHEET S001 FOR GENERAL STRUCTURAL NOTES.

SYMBOL	DATE	REVISION
C		
D		

SYMBOL	DATE	REVISION
A		
B		

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ISSUE DATE: 07/26/2023
COMP FILE No: F13633
START PROJECT No: 61437C

DESIGNED: CPH
DRAWN BY: CPH
REVIEWED BY: CPH

PROFESSIONAL REPRESENTATION
JOSE MIGUEL ORTIZ
LICENSE No. 67920
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SHEET TITLE: FOUNDATION AND FRAMING PLAN

PROJECT: CONSTITUTION CONVENTION STATE PARK
PORT ST. JOE / GULF COUNTY / FLORIDA

SHEET NO.
S100

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION

ROOF / SHEATHING NAILING SCHEDULE

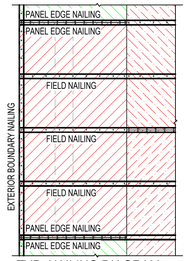
LOCATION	SIZE	SPACING
BOUNDARY	10d RING SHANK	4"
PANEL EDGE	10d RING SHANK	4"
FIELD	10d RING SHANK	6"

CORROSION PROTECTION

ALL HURRICANE CONNECTORS, STRAPS THEIR FASTENERS ARE TO BE PAINTED WITH ONE COAT OF ONE OF THE FOLLOWING CORROSION PROTECTION PRODUCTS:

- COAL TAR EPOXY-POLYAMIDE (SSPC-PAINT 16)
- EPOXY-POLYAMIDE PRIMER (SSPC-PAINT 22)

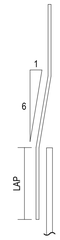
FOLLOW ALL MANUFACTURER'S DIRECTIONS ON PREPING AND CLEANING THE MATERIAL SURFACES PRIOR TO PAINT APPLICATION



TYP. NAILING DIAGRAM

I1 — ROOF SHEATHING DETAIL

1/4" = 1'-0"



CONCRETE STRENGTH	fc = 3000 PSI				fc = 4000 PSI			
CLASS OF LAP SPLICE	CLASS "A"		CLASS "B"		CLASS "A"		CLASS "B"	
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	1'-10"	1'-5"	2'-4"	1'-10"	1'-7"	1'-3"	2'-1"	1'-7"
#4	2'-5"	1'-10"	3'-1"	2'-5"	2'-1"	1'-7"	2'-9"	2'-1"
#5	3'-0"	2'-4"	3'-11"	3'-0"	2'-7"	2'-0"	3'-5"	2'-7"
#6	3'-7"	2'-9"	4'-8"	3'-7"	3'-1"	2'-5"	4'-1"	3'-1"
#7	5'-3"	4'-0"	6'-9"	5'-2"	4'-6"	3'-6"	5'-11"	4'-6"
#8	6'-0"	4'-7"	7'-9"	6'-0"	5'-2"	4'-0"	6'-9"	5'-2"
#9	6'-9"	5'-2"	8'-9"	6'-9"	5'-10"	4'-6"	7'-7"	5'-10"
#10	7'-7"	5'-10"	9'-10"	7'-7"	6'-7"	5'-1"	8'-6"	6'-7"
#11	8'-5"	6'-6"	10'-11"	8'-5"	7'-3"	5'-7"	9'-5"	7'-3"

NOTES:

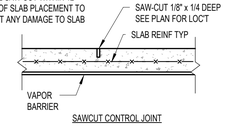
1. UNLESS INDICATED OTHERWISE, USE CLASS "B" LAP SPLICE LENGTHS, MULTIPLIED BY THE APPLICABLE FACTORS(S) LISTED BELOW.
2. WHERE THE CLEAR SPACING OF BARS BEING SPLICED IS LESS THAN 2 BAR DIAMETERS, INCREASE THE LAP LENGTH BY 50%.
3. WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER, INCREASE THE LAP LENGTH BY 50%
4. A CLASS "A" SPLICE MAY BE USED ONLY WHERE NOTED ON THE DRAWINGS. WHERE DEVELOPMENT LENGTH (Ld) IS REQUIRED OR CALLED OUT ON THE DRAWINGS, USE CLASS "A" LAP SPLICE LENGTH.
5. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
6. LAP SPLICE LENGTHS IN TABLE ARE FOR NORMAL WEIGHT CONCRETE. WHERE LIGHTWEIGHT AGGREGATE CONCRETE IS USED, INCREASE LAP SPLICE LENGTH BY 30%
7. SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS SHALL BE STAGGERED.
8. SPLICES OF HORIZONTAL REINFORCEMENT IN WALLS CONTAINING TWO CURTAINS OF REINFORCEMENT SHALL NOT OCCUR IN THE SAME LOCATION.

I6 — REBAR OFFSET & LAP SPLICE REQUIREMENTS

1" = 1'-0"

CONSTRUCTION JOINT (IF REQ'D)

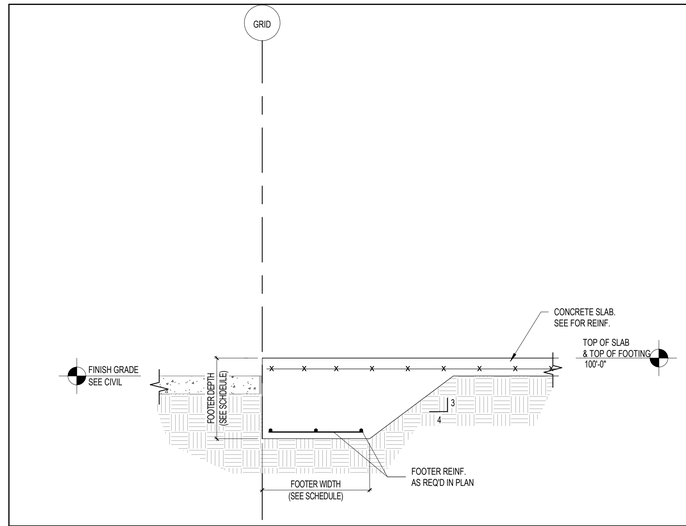
NOTE:
1. PROVIDE SAW-CUT WITHIN 12 HOURS OF SLAB PLACEMENT TO PREVENT ANY DAMAGE TO SLAB



SAWCUT CONTROL JOINT

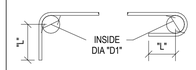
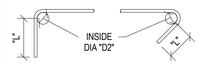
C1 — SLAB ON GRADE JOINTS

3/4" = 1'-0"



C6 — MONOLITHIC FOOTER

3/4" = 1'-0"

	MAIN REINFORCEMENT			STIRRUPS & TIES		
						
BAR SIZE	90° HOOK LENGTH "L"	INSIDE DIA. "D1"	180° HOOK LENGTH "L"	90° HOOK LENGTH "L"	INSIDE DIA. "D2"	135° HOOK LENGTH "L"
#3	4 1/2"	2 1/4"	2 1/2"	3"	1 1/2"	3"
#4	6"	3"	2 1/2"	3"	2"	3"
#5	7 1/2"	3 3/4"	2 1/2"	3 3/4"	2 1/2"	3 3/4"
#6	9"	4 1/2"	3"	9"	4 1/2"	4 1/2"
#7	10 1/2"	5 1/4"	3 1/2"	10 1/2"	5 1/4"	5 1/4"
#8	1'-0"	6"	4"	1'-0"	6"	6"
#9	1'-1 1/2"	9 1/2"	4 1/2"	-	-	-
#10	1'-3 1/4"	10 3/4"	5 1/4"	-	-	-
#11	1'-5"	1'-0"	5 3/4"	-	-	-

I11 — REBAR STANDARD HOOKS & BENDS

1" = 1'-0"

Department of Environmental Protection
Division of Recreation and Parks
Bureau of Design and Construction
3800 Commonwealth Blvd., Tallahassee, FL 32399 (851) 425-2300

SYMBOL	DATE	REVISION
A		
B		
C		
D		

DESIGNER: Designer
DRAWN BY: Author
REVIEWED BY: Checker
Consultant:

cph
A Full Service A & E Firm
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ISSUE DATE: 07/26/2023
COMP FILE No: F13633
START PROJECT No: 61437C

PROFESSIONAL REGISTRATION
JOSE MIGUEL ORTIZ
LICENSE No. 67920
STATE OF FLORIDA
PROFESSIONAL ENGINEER

CONSTITUTION CONVENTION STATE PARK

SHEET TITLE: TYPICAL SECTIONS AND DETAILS

PROJECT TITLE: CONSTITUTION CONVENTION STATE PARK
PORT ST. JOE / GULF COUNTY / FLORIDA

SHEET NO.
S200

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION