



September 1, 2026

Cindy Mulkey, Deputy Director
Office of Siting Coordination
Department of Environmental Protection
2600 Blair Stone Road, MS 5500
Tallahassee, Florida 32399-2400

Re: Florida Power & Light Company
Martin Power Plant (PA 89-27)
Request for Post Certification Amendment
Martin Plant 500kV Laydown Yard Improvements

Dear Ms. Mulkey:

Florida Power & Light Company (FPL) requests a post-certification amendment to the Site Certification for FPL's Martin Power Plant, pursuant to Section 403.5113, Florida Statutes, and Rule 62-17.2015(1), Florida Administrative Code. The purpose of this amendment request is for authorization to redevelop a portion of the former Martin Solar Energy Center into stabilized laydown yard areas.

The proposed improvements are located within the previously permitted Martin Solar Energy Center, which lies within the existing Site Certification Area (SCA) of the Martin Power Plant. The former solar energy facility was decommissioned in 2024, and the associated solar panels and supporting infrastructure have been removed. The existing solar basins and associated stormwater management infrastructure remain in place and are proposed to be repurposed to support future operational and construction activities at the Martin Power Plant.

The project consists of the redevelopment of Solar Basins S2 and S3 and portions of Solar Basins S7 and S8 into stabilized laydown yard areas. The laydown yards will consist primarily of compacted native material to provide durable equipment and material storage areas. Aggregate access drives will also be constructed to provide vehicular access to the proposed laydown areas.

A stormwater management analysis was performed for the proposed improvements within Solar Basins S2, S3, S7, and S8. The evaluation was limited to the solar basins affected by the proposed work, with the remaining portions of the existing stormwater management system remaining undisturbed. The stormwater management performance at the existing modified control structure for each affected basin was evaluated to confirm that post-development water quality treatment, stage-storage performance, and discharge rates are maintained consistent with the existing condition.

The analysis demonstrates that the proposed improvements maintain the required stormwater management function of the affected solar basins. Post-development discharge rates from the affected basins do not exceed the corresponding existing-condition discharge rates, and the proposed improvements do not increase the quantity of stormwater discharged to downstream drainage basins. Accordingly, the proposed SCA Amendment will not increase the hydraulic loading on downstream stormwater management facilities or receiving systems.

Detailed supporting calculations, basin information, and stormwater management analyses are provided in the Stormwater Management Report (*Attachment C*).

The proposed activities will maintain compliance with the Conditions of Certification and applicable FDEP regulations, therefore no changes to the existing conditions of certification are required. No wetlands, surface waters, or listed threatened or endangered species will be impacted by this project.

Thank you for your consideration of FPL's request to amend Martin Clean Energy Center Site Certification No. PA 89-27. If you have any questions regarding this submittal, please do not hesitate to contact me at 561-308-9929 or Daniel.lambert@fpl.com.

Sincerely,

A handwritten signature in black ink, appearing to read "Daniel Lambert", with a stylized, flowing script.

Daniel Lambert
Senior Environmental Specialist, PWS

CC: Nate Senn, FDEP
Ken Proctor, FPL
Victoria Torres, FPL

Attachments:

Attachment A – Project Area Location Map
Attachment B – Permit Drawings
Attachment C – Stormwater Report

Attachment A
Project Area Location Map

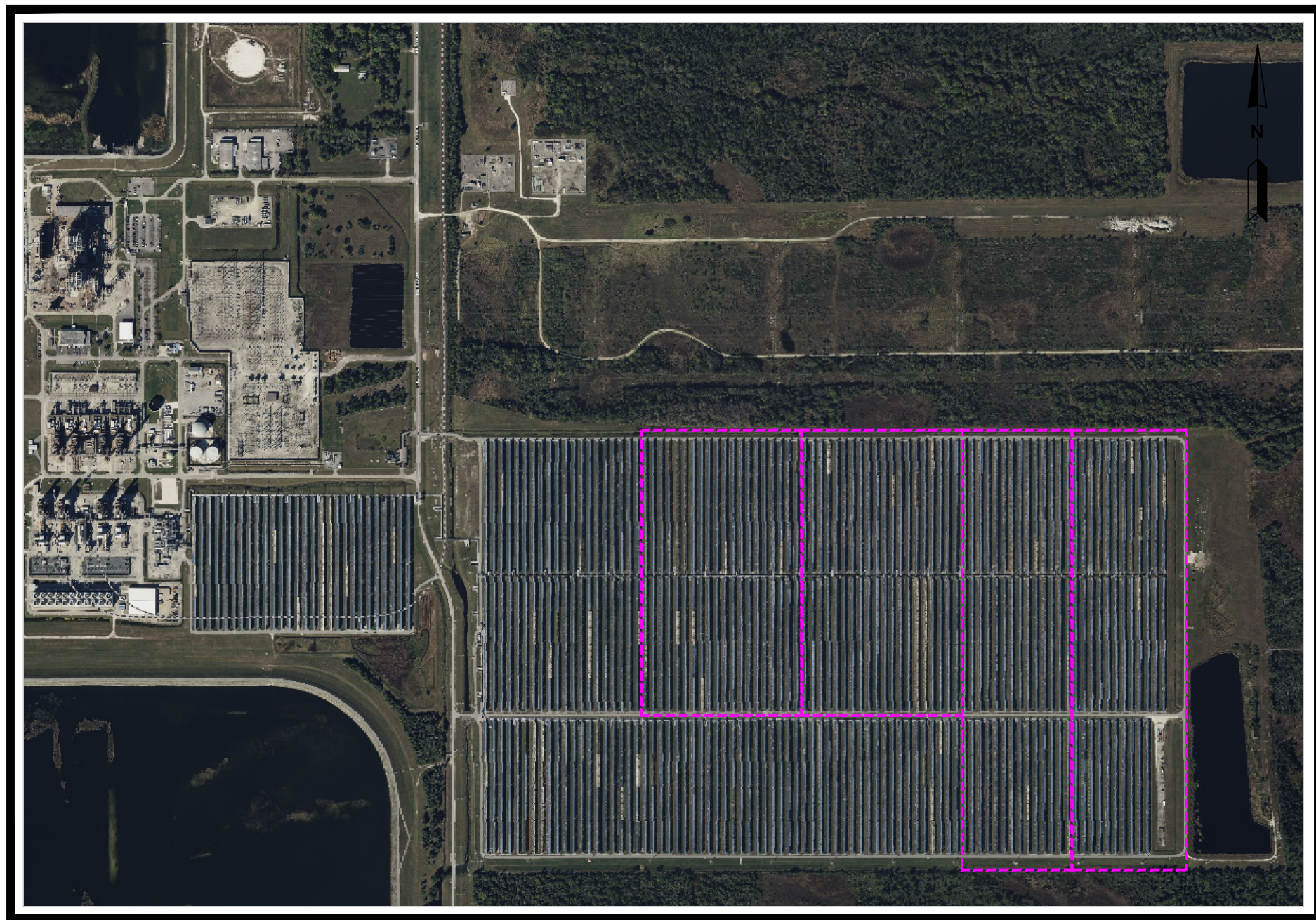
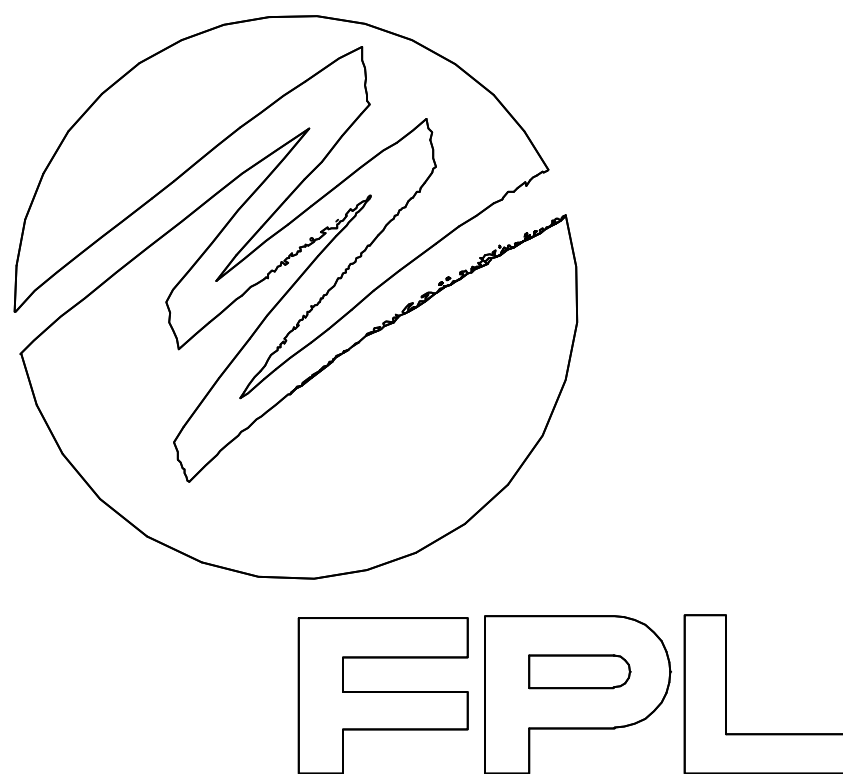


FIGURE 1 - SITE LOCATION MAP
FPL MARTIN PLANT 500kV LAYDOWN YARD

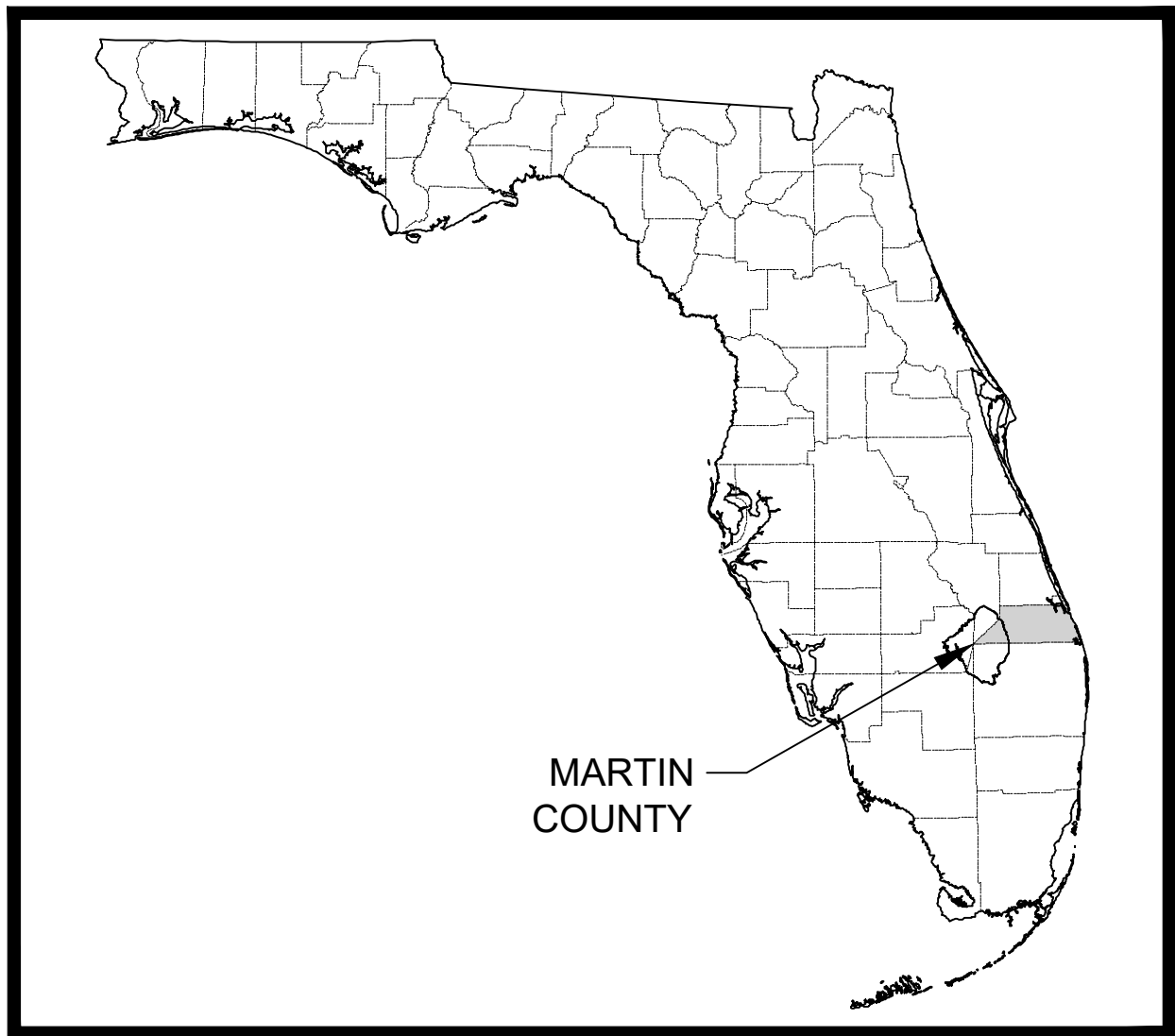
Attachment B
Permit Drawings

FLORIDA POWER & LIGHT COMPANY

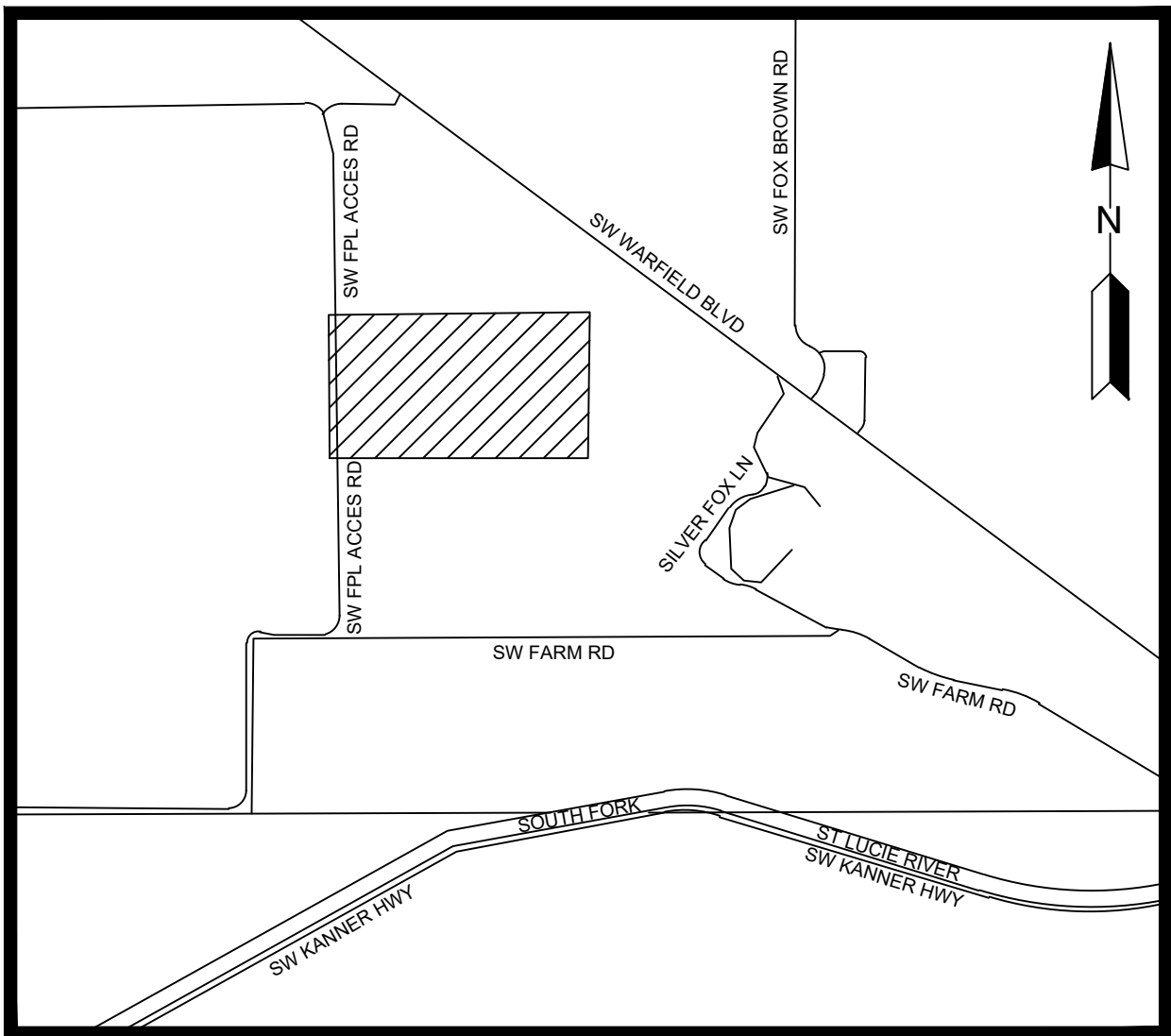
MARTIN PLANT - 500kV LAYDOWN YARD



21900 SW WARFIELD BOULEVARD, INDIANTOWN, FLORIDA
MARTIN COUNTY
SECTION 28/29, TWP. 39 S., RGE. 38 E.



LOCATION MAP
(NOT TO SCALE)



VICINITY MAP
(NOT TO SCALE)

PROJECT DATA

SITE ADDRESS: 21900 SW WARFIELD BOULEVARD, INDIANTOWN, FLORIDA

FOLIO: 30-39-38-000-00010-3 AND 31-39-38-000-00010-1

ZONING DISTRICT: U - UTILITY (VILLAGE OF INDIANTOWN)

PROJECT DATUM: NORTH AMERICAN VERTICAL DATUM (NAVD) 1988.

FLOOD ZONE: THIS PROPERTY LIES IN FLOOD ZONE "X" ACCORDING TO FEMA FLOOD MAP (FIRM) PANEL NUMBER 12085C0250G, EFFECTIVE DATE: MARCH 16, 2015.

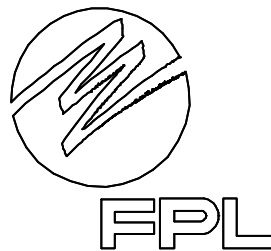
BUILDING CODE: FLORIDA BUILDING CODE, 8TH EDITION (2023)

PREPARED BY:



SOMI ENGINEERING, LLC
2332 GALIANO STREET 2ND FLOOR
CORAL GABLES, FL 33134
PH. (561) 901-9255
FLORIDA CERTIFICATE OF AUTHORIZATION NO.32647
SOMI PROJECT NO. 26008

PREPARED ON BEHALF OF:

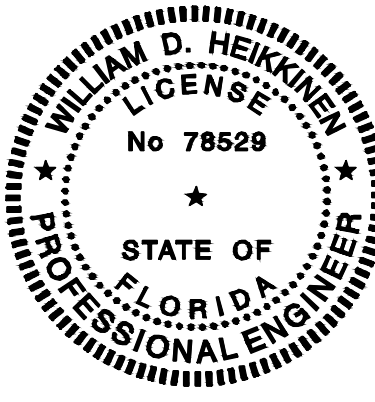


FLORIDA POWER AND LIGHT COMPANY
TRANSMISSION AND SUBSTATION SITING SECTION
700 UNIVERSE BLVD. (TS4-JW)
JUNO BEACH, FLORIDA 33408

INDEX OF DRAWINGS

INDEX NO.	SHEET NO.	DESCRIPTION
E-MAR00	1 OF 1	COVER SHEET
E-MAR01J	1 OF 2	GENERAL NOTES AND SPECIFICATIONS
E-MAR01J	2 OF 2	JURISDICTIONAL SITE PLAN
E-MAR02	1 OF 11	DEMOLITION PLAN
E-MAR02	2 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	3 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	4 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	5 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	6 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	7 OF 11	FENCE FILL AND GRADE PLAN
E-MAR02	8 OF 11	FENCE FILL AND GRADE SECTIONS
E-MAR02	9 OF 11	FENCE FILL AND GRADE DETAILS
E-MAR02	10 OF 11	STORMWATER POLLUTION PREVENTION PLAN
E-MAR02	11 OF 11	STORMWATER POLLUTION PREVENTION PLAN AND DETAILS

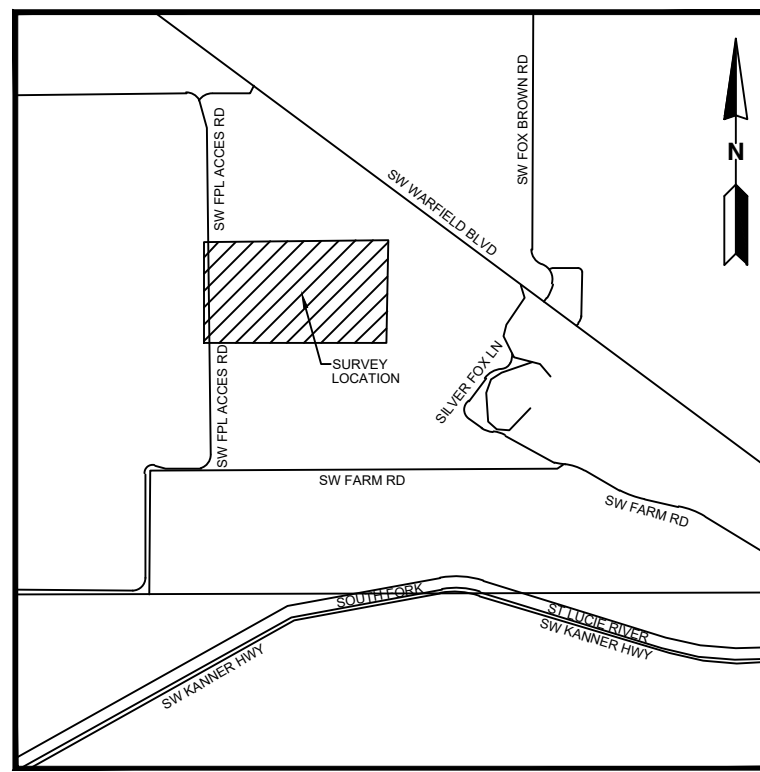
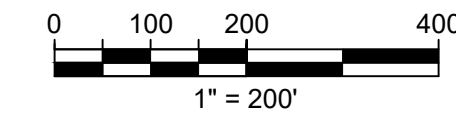
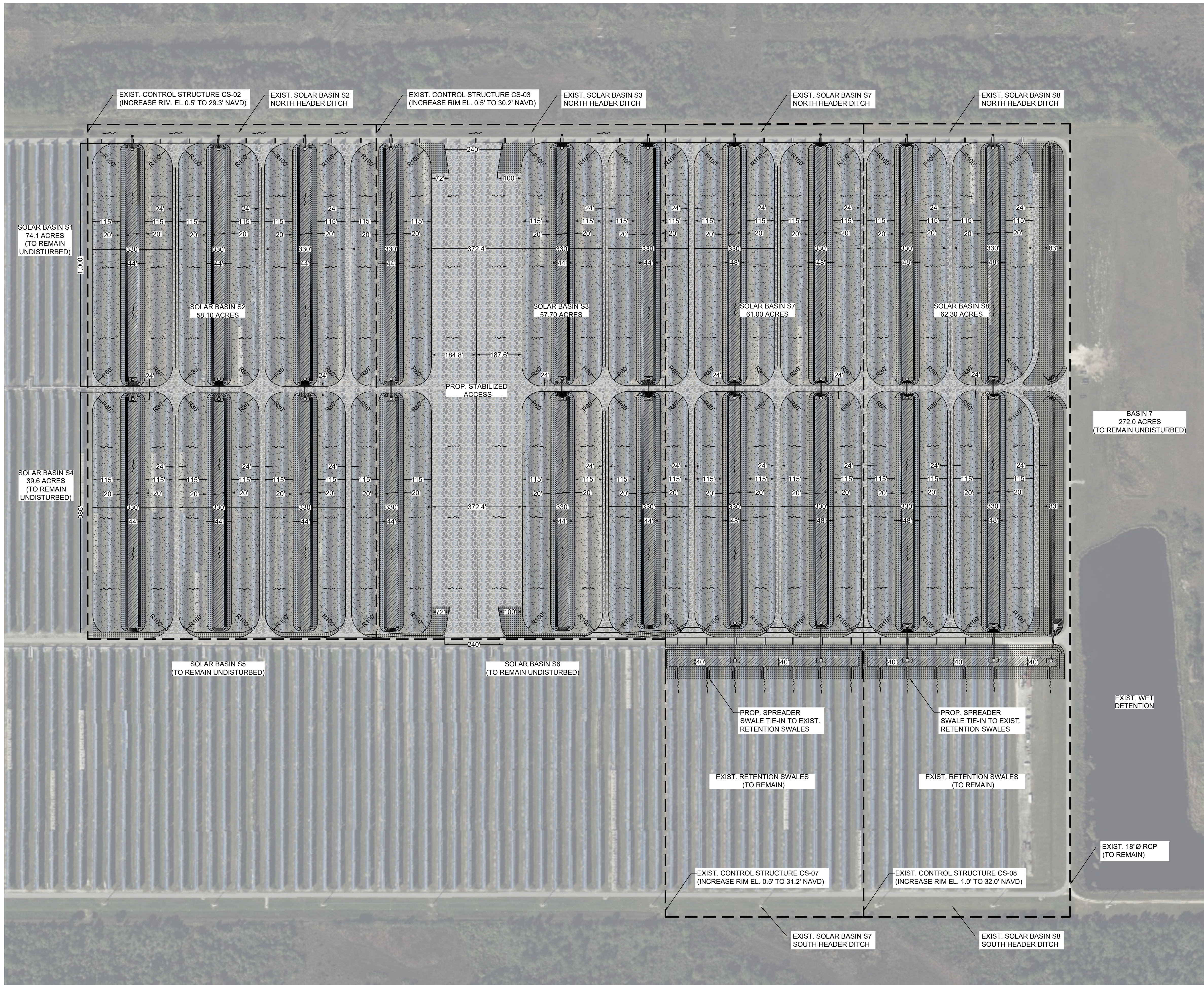
THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM D. HEIKKINEN, P.E. ON AUGUST 18, 2026.
PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.



PERMIT DRAWINGS
DATE: AUGUST 18, 2026



MARTIN PLANT - 500kV LAYDOWN YARD	
DATE: AUG 2026	E-MAR00
CADE	



LOCATION SKETCH
SECTION 28/29, TOWNSHIP 39 SOUTH, RANGE 38 EAST
MARTIN COUNTY, FLORIDA
(NOT TO SCALE)

LEGEND:

- PROJECT LIMITS/BASIN BOUNDARY
- EXISTING ELEVATION, FT NAVD
- PROPOSED CONTOUR, FT NAVD
- PROPOSED COMPACTED NATIVE MATERIAL (LAYDOWN AREA)
- PROPOSED BOTTOM OF DRY RETENTION SWALE (SEED AND MULCH)
- PROPOSED SOD COVER (DRY RETENTION SWALE SLOPES ONLY)
- PROPOSED SEED AND MULCH
- PROPOSED STABILIZED ACCESS
- PROPOSED ASPHALT
 - 3" TYPE SP STRUCTURAL COURSE ASPHALT
 - 9" LIMEROCK BASE COMPACTED TO 98%, LBR 100
 - 12" SUB-BASE COMPACTED TO 98%, LBR 40 MIN.

GENERAL NOTES:

- APPLICANT AND AGENT: VICTORIA TORIS FOR FLORIDA POWER AND LIGHT, OWNER & DEVELOPER, 700 UNIVERSE BLVD., TS4/JW, JUNO BEACH, FLORIDA 33408. PHONE 561-904-3669.
- FPL CERTIFIES THAT THIS FACILITY WILL BE USED ONLY BY EMPLOYEES, AND THE WORK ASSOCIATED WITH HIGH VOLTAGE ELECTRICAL EQUIPMENT CANNOT REASONABLY BE PERFORMED BY HANDICAPPED PERSONS. PURSUANT TO CHAPTER 553.504(1), OF FLORIDA STATUTES, THIS FACILITY IS NOT SUBJECT TO THE ACCESSIBILITY REQUIREMENTS OF THE ADA.
- THIS FACILITY WILL CONSUME NO UNRECOVERABLE ENERGY AND IS EXEMPT FROM THE FLORIDA ENERGY REVIEW CODE.
- THIS FACILITY WILL GENERATE NO SOLID WASTE.
- THIS IS AN UNMANNED RESTRICTED ACCESS FACILITY. NO FPL EMPLOYEES REPORT TO THIS LOCATION. THEREFORE, NO PARKING SPACES WILL BE PROVIDED.
- THIS PROPERTY LIES IN FLOOD ZONE "X" (ACCORDING TO FEMA FLOOD MAP (FIRM) PANEL NUMBER 12085C0250G, EFFECTIVE DATE: MARCH 16, 2015 (NOT PRINTED).
- SITE ADDRESS: 21900 SW WARFIELD BOULEVARD, INDIANTOWN, FLORIDA
- PARCEL ID.: 30-39-38-000-000-0010-3 AND 31-39-38-000-000-00010-1
- AS PER FLORIDA BUILDING CODE 2010 SECTION 102.2(F): STRUCTURES OR FACILITIES OF ELECTRICAL UTILITIES, AS DEFINED IN SECTION 366.02, FLORIDA STATUTES, WHICH ARE DIRECTLY INVOLVED IN THE GENERATION, TRANSMISSION, OR DISTRIBUTION OF ELECTRICITY ARE EXEMPT FROM THE FLORIDA BUILDING CODE AS PROVIDED BY LAW, AND ANY FURTHER EXEMPTIONS SHALL BE AS DETERMINED BY THE LEGISLATOR AND PROVIDED BY LAW.

SURVEYOR'S DESCRIPTION:

A PORTION OF LAND LYING IN SECTIONS 28 AND 29, TOWNSHIP 39 SOUTH, RANGE 38 EAST, MARTIN COUNTY, FLORIDA, AND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHEAST CORNER OF SAID SECTION 28; THENCE SOUTH 89°47'48" WEST ALONG THE SOUTH LINE OF SAID SECTION 28, A DISTANCE OF 3,000.02 FEET; THENCE NORTH 00°23'19" WEST, ALONG A LINE PARALLEL WITH AND 3,000.02 FEET WEST OF THE EAST LINE OF SAID SECTION 28, A DISTANCE OF 4,243.54 FEET; THENCE SOUTH 89°58'57" EAST, ALONG A LINE PARALLEL WITH AND 1092.00 FEET SOUTH OF THE NORTH LINE OF SAID SECTION 29, A DISTANCE OF 3,000.08 FEET TO A POINT ON THE WEST LINE OF SAID SECTION 28; THENCE NORTH 89°23'26" EAST ALONG A LINE 1,092.00 FEET SOUTH AND PARALLEL WITH THE NORTH LINE OF SAID SECTION 28, A DISTANCE OF 4,688.39 FEET; THENCE SOUTH 00°44'14" WEST ALONG A LINE PARALLEL WITH AND 700 FEET WEST OF THE EAST LINE OF SAID SECTION 28, A DISTANCE OF 4,257.75 FEET TO A POINT ON THE SOUTH LINE OF SAID SECTION 28; THENCE SOUTH 89°41'50" WEST, ALONG SAID SOUTH LINE, A DISTANCE OF 4,604.68 FEET TO THE POINT OF BEGINNING.

SAID LANDS LYING IN MARTIN COUNTY, FLORIDA. CONTAINING 32,434,963 SQUARE FEET (744.604 ACRES), MORE OR LESS.



SOMI ENGINEERING, LLC
2332 GALIANO STREET 2ND FLOOR, CORAL GABLES, FL 33134
P: (561) 901-9255
FLORIDA CERTIFICATE OF AUTHORIZATION NO. 32647

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026SCALE: AS NOTED



DR: N/AER: WDH
CH: WDHCOR: N/A
APPROVED:

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
JURISDICTIONAL SITE PLAN

PERMIT DRAWINGS
DATE: AUGUST 18, 2026

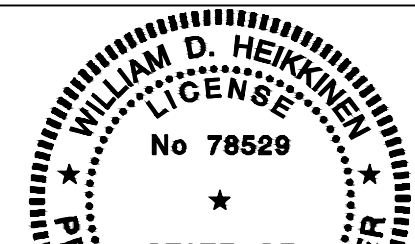
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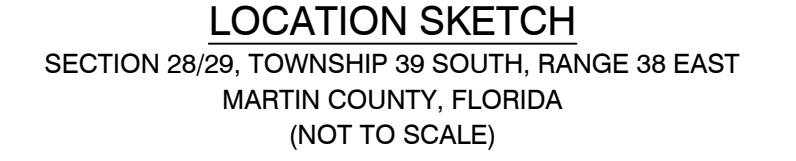
SHEET 2 OF 2
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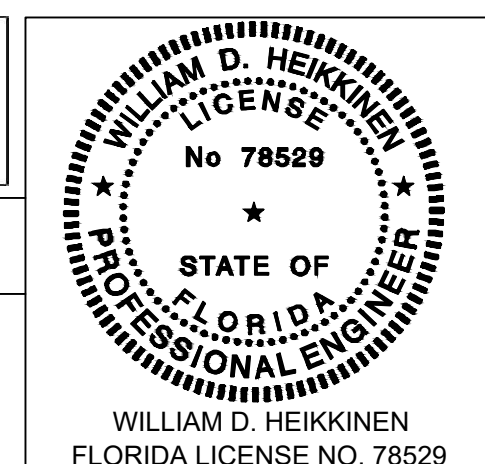
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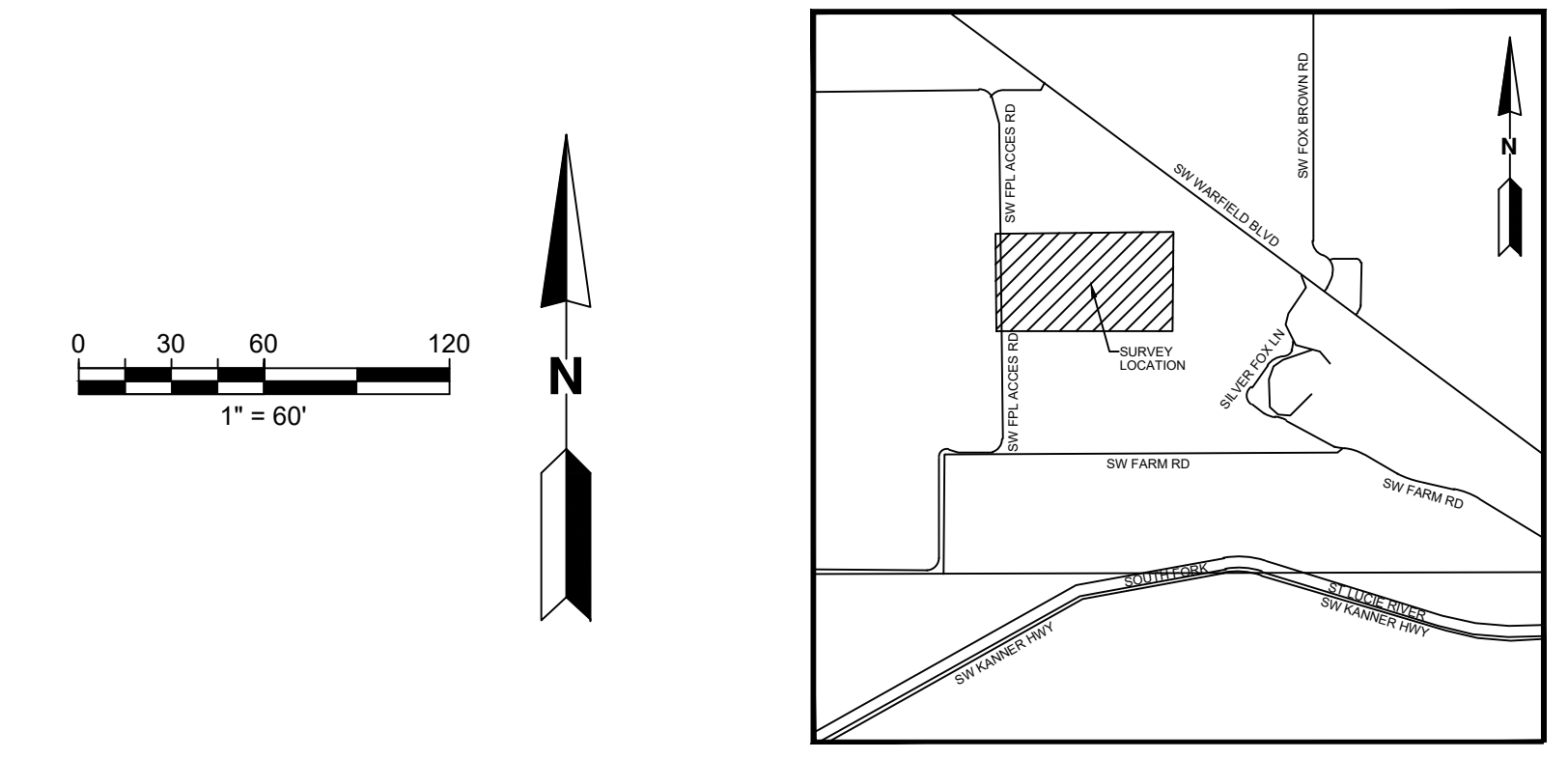
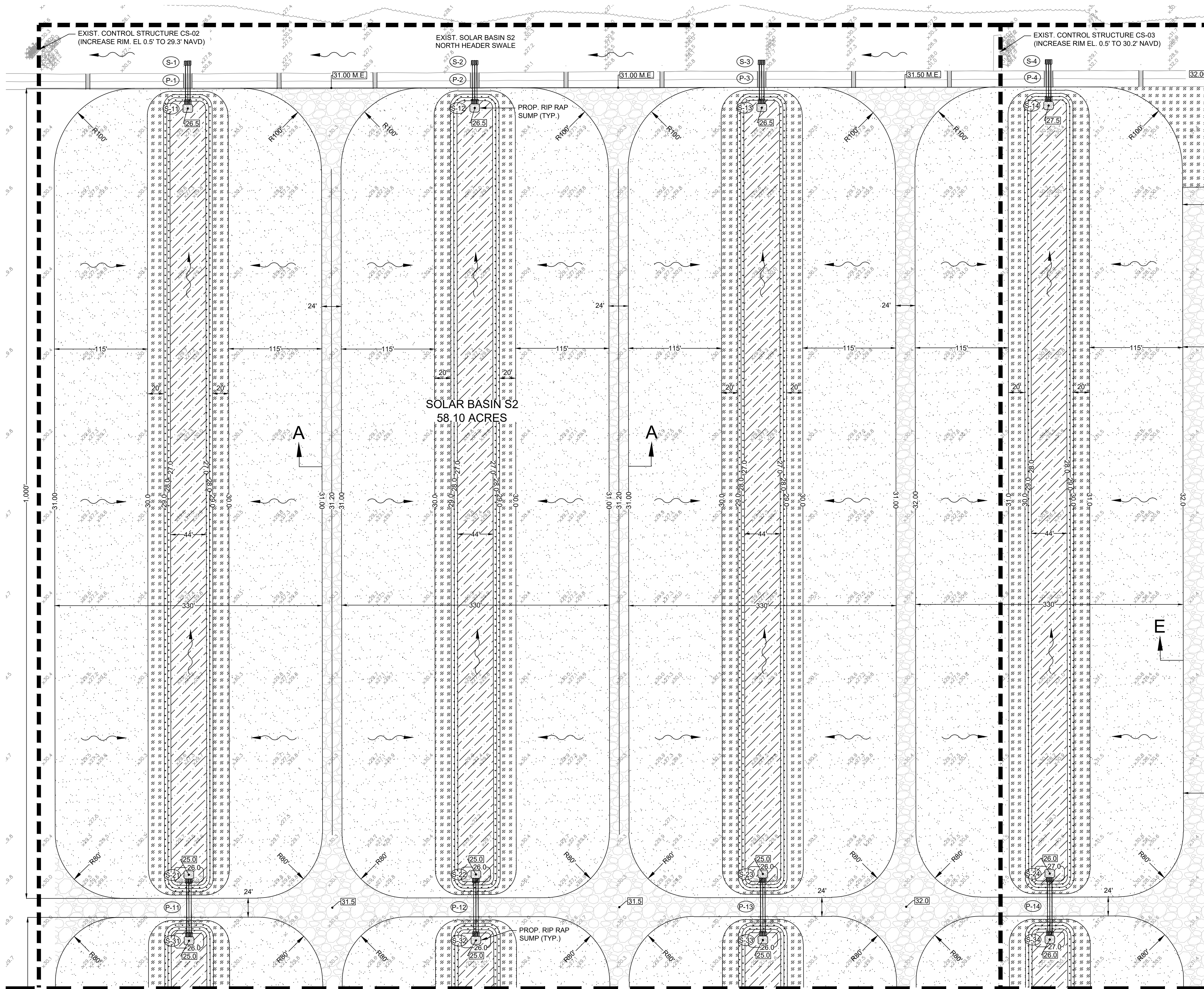

Call 811 or visit sunshinefl.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!


WILLIAM D. HEIKKINEN
FLORIDA LICENSE NO. 78529
FLORIDA LICENSE NO. 78529



*CONTRACTOR SHALL BE RESPONSIBLE FOR THEIR OWN DEMOLITION QUANTITIES TAKEOFF.
ENGINEER'S QUANTITIES ARE FOR INFORMATIONAL PURPOSES ONLY.





LEGEND:

- PROJECT LIMITS/BASIN BOUNDARY
- EXISTING ELEVATION, FT NAVD
- PROPOSED CONTOUR, FT NAVD
- PROPOSED COMPACTED NATIVE MATERIAL (LAYDOWN AREA)
- PROPOSED BOTTOM OF DRY RETENTION SWALE (SEED AND MULCH)
- PROPOSED SOD COVER (DRY RETENTION SWALE SLOPES ONLY)
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 - 12" SUB-BASE COMPACTED TO 98%, LBR 40 MIN.
- PROPOSED ELEVATION, FT NAVD
- PROPOSED MATCH EXISTING ELEVATION, FT NAVD
- PROPOSED DRAINAGE STRUCTURE/PIPE CALLOUTS

GENERAL NOTES:

- [X.XX] DENOTES LOCATION AND ELEVATION OF PROPOSED GRADE IN NAVD 88.
- ELEVATIONS WHICH INDICATE TOP OF FILL OR LIMEROCK WHICH ARE TO BE COVERED WITH LOOSE ROCK OR ASPHALTIC CONCRETE DO NOT INCLUDE 0.17' (2") FOR LOOSE ROCK OR 0.17' (2") FOR ASPHALTIC CONCRETE AND ARE SHOWN AS [X.XX]
- ELEVATIONS WHICH INDICATE TOP OF FILL OR CUT THAT ARE TO BE SODDED ARE THE PROPOSED ELEVATIONS OF THE TOP OF SOD AND ARE SHOWN [X.X]
- ← INDICATES THE DIRECTION OF FLOW OF STORM WATER RUNOFF.
- ALL CLEARING, EXCAVATING, FILLING, GRADING, PAVING AND DRAINAGE SHALL BE PERFORMED IN CONFORMANCE WITH FPL D-2 SPECIFICATION.
- FPL SHALL ASSUME RESPONSIBILITY FOR MAINTENANCE OF THE STORMWATER FACILITIES WITHIN FPL PROPERTY UPON COMPLETION AND ACCEPTANCE BY THE COMPANY REPRESENTATIVE.
- STORMWATER MANAGEMENT DESIGN DATA:
 - STORMWATER PROJECT AREA (SOLAR BASINS S2, S3, S7, S8) = 239.10 ACRES
 - IMPERVIOUS AREA (STABILIZED ACCESS, ASPHALT) = 32.73 ACRES
 - PERVIOUS AREA (COMPACTED NATIVE MATERIAL, DRY RETENTION SWALES) = 206.37 ACRES
- THE LOCATION OF EXISTING UTILITIES SHOWN IS APPROXIMATE ONLY AND MUST BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO BEGINNING WORK.
- FIELD TESTING
 - A. PAVING SUBGRADE TO BE TESTED FOR DENSITY EVERY 10,000 SQ. FT. OR PORTION THEREOF AND ONE LBR SHALL BE TAKEN IN AREA OF ASPHALT.
 - B. PAVING BASE AND YARD BASE TO BE TESTED FOR DENSITY EVERY 10,000 SQ. FT. OR PORTION THEREOF. PIT CERTIFICATION (FDOT) WILL BE SATISFACTORY FOR EVIDENCE OF LBR.
- APPLICANT AND AGENT: VICTORIA TORIS FOR FLORIDA POWER AND LIGHT, OWNER & DEVELOPER, 700 UNIVERSE BLVD., TS4/JW. JUNO BEACH, FLORIDA 33408. PHONE 561-904-3669.
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- HIS FACILITY WILL CONSUME NO UNRECOVERABLE ENERGY AND IS EXEMPT FROM THE FLORIDA ENERGY REVIEW CODE.
- THIS FACILITY WILL GENERATE NO SOLID WASTE.
- THIS IS AN UNMANNED RESTRICTED ACCESS FACILITY. NO FPL EMPLOYEES REPORT TO THIS LOCATION AND MAINTENANCE CREWS WILL PARK INSIDE THE FENCED AREA. THEREFORE, NO PARKING SPACES WILL BE PROVIDED.
- THIS PROPERTY LIES IN FLOOD ZONE "X" (ACCORDING TO FEMA FLOOD MAP (FIRM) PANEL NUMBER 12085C0250G, EFFECTIVE DATE: MARCH 16, 2015 (NOT PRINTED).
- ELEVATIONS WHICH INDICATE PROPOSED ASPHALT PAVEMENT OR TO MATCH EXISTING ASPHALT PAVEMENT ELEVATION REFER TO TOP OF ASPHALTIC CONCRETE ELEVATION.

SOMI ENGINEERING, LLC
2332 GALIANO STREET 2ND FLOOR, CORAL GABLES, FL 33134
P: (561) 901-9255
FLORIDA CERTIFICATE OF AUTHORIZATION NO. 32647

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026 SCALE: AS NOTED

POWER DELIVERY

DR: N/A ER: WDH
CH: WDH COR: N/A
APPROVED:

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA

FENCE FILL AND GRADE PLAN

SOMI NO. 26008 FPL NO.

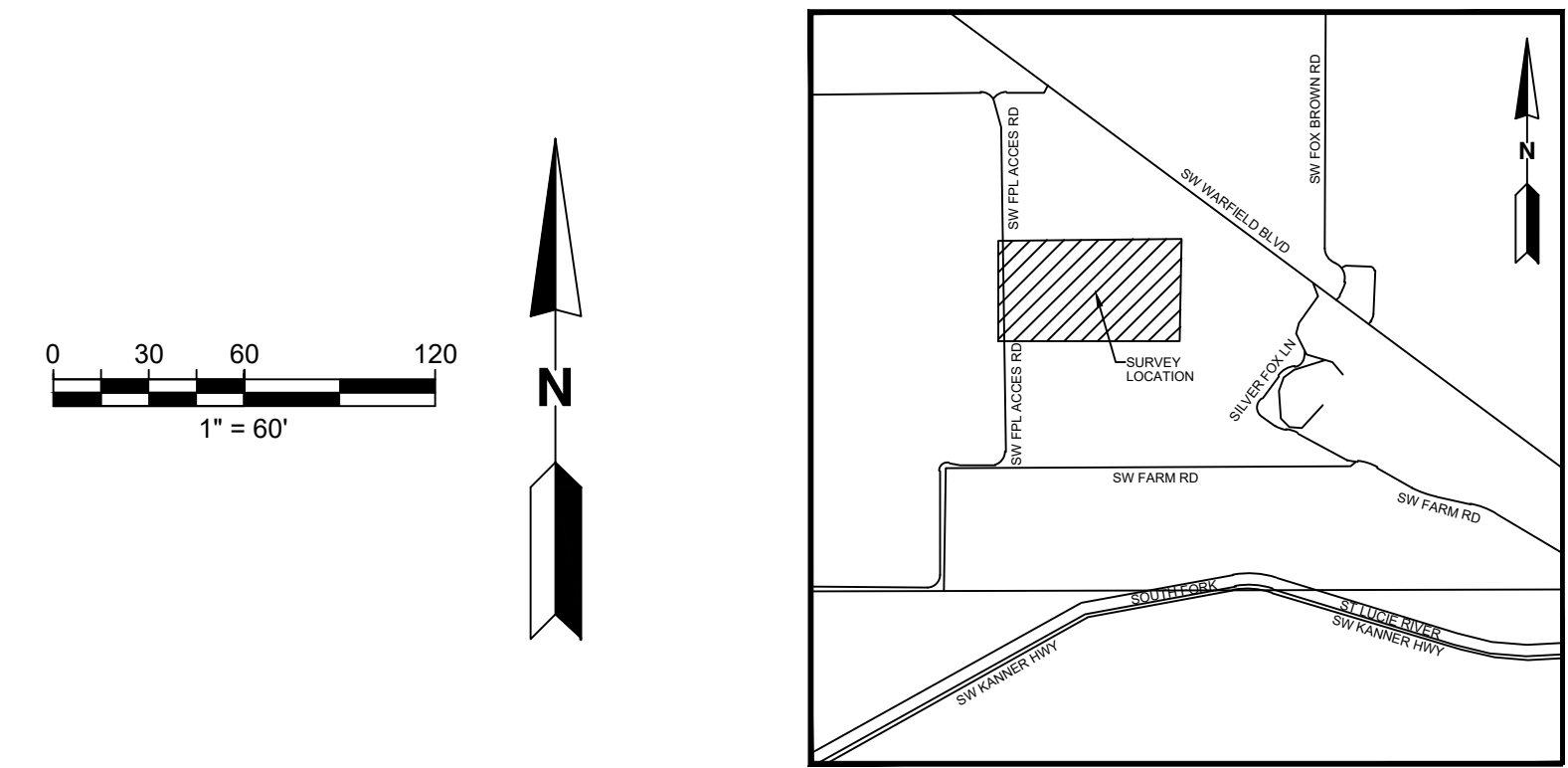
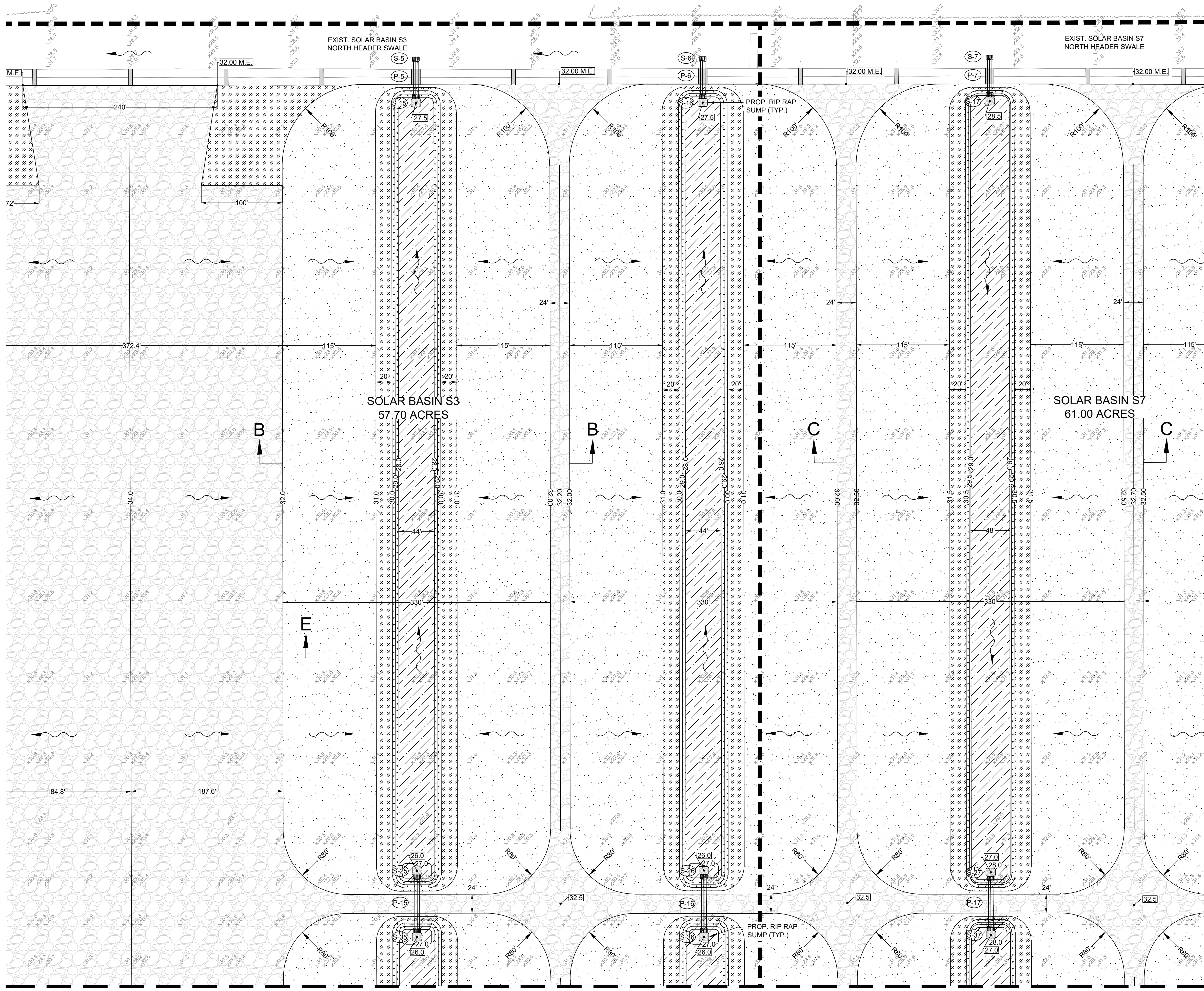
SHEET 2 OF 11
E-MAR02

REV NO.

PERMIT DRAWINGS
DATE: AUGUST 18, 2026

Sunshine
Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked. Check positive response codes before you dig!

WILLIAM D. HEIKKINEN
No 78529
STATE OF FLORIDA
PROFESSIONAL ENGINEER
WILLIAM D. HEIKKINEN
FLORIDA LICENSE NO. 78529



LEGEND:

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- PROPOSED DRAINAGE STRUCTURE/PIPE CALLOUTS

MITERED END SECTION SCHEDULE:

STR NO.	STRUCTURE TYPE	FDOT INDEX	INVERT ELEVATION				NOTES
			N	S	E	W	
S-1	MITERED END SECTION	430-021	-	26.5	-	-	3 COUNT
S-2	MITERED END SECTION	430-021	-	26.5	-	-	3 COUNT
S-3	MITERED END SECTION	430-021	-	26.5	-	-	3 COUNT
S-4	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-5	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-6	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-7	MITERED END SECTION	430-021	-	28.5	-	-	3 COUNT
S-8	MITERED END SECTION	430-021	-	28.5	-	-	3 COUNT
S-9	MITERED END SECTION	430-021	-	29.0	-	-	3 COUNT
S-10	MITERED END SECTION	430-021	-	29.0	-	-	3 COUNT
S-11	MITERED END SECTION	430-021	26.5	-	-	-	3 COUNT
S-12	MITERED END SECTION	430-021	26.5	-	-	-	3 COUNT
S-13	MITERED END SECTION	430-021	26.5	-	-	-	3 COUNT
S-14	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-15	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-16	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-17	MITERED END SECTION	430-021	28.5	-	-	-	3 COUNT
S-18	MITERED END SECTION	430-021	28.5	-	-	-	3 COUNT
S-19	MITERED END SECTION	430-021	29.0	-	-	-	3 COUNT
S-20	MITERED END SECTION	430-021	29.0	-	-	-	3 COUNT
S-21	MITERED END SECTION	430-021	-	25.0	-	-	3 COUNT
S-22	MITERED END SECTION	430-021	-	25.0	-	-	3 COUNT
S-23	MITERED END SECTION	430-021	-	25.0	-	-	3 COUNT
S-24	MITERED END SECTION	430-021	-	26.0	-	-	3 COUNT
S-25	MITERED END SECTION	430-021	-	26.0	-	-	3 COUNT
S-26	MITERED END SECTION	430-021	-	26.0	-	-	3 COUNT
S-27	MITERED END SECTION	430-021	-	27.0	-	-	3 COUNT
S-28	MITERED END SECTION	430-021	-	27.0	-	-	3 COUNT
S-29	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-30	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-31	MITERED END SECTION	430-021	25.0	-	-	-	3 COUNT
S-32	MITERED END SECTION	430-021	25.0	-	-	-	3 COUNT
S-33	MITERED END SECTION	430-021	25.0	-	-	-	3 COUNT
S-34	MITERED END SECTION	430-021	26.0	-	-	-	3 COUNT
S-35	MITERED END SECTION	430-021	26.0	-	-	-	3 COUNT
S-36	MITERED END SECTION	430-021	26.0	-	-	-	3 COUNT
S-37	MITERED END SECTION	430-021	27.0	-	-	-	3 COUNT
S-38	MITERED END SECTION	430-021	27.0	-	-	-	3 COUNT
S-39	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-40	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-41	MITERED END SECTION	430-021	-	29.5	-	-	1 COUNT
S-42	MITERED END SECTION	430-021	29.5	-	-	-	1 COUNT
S-43	MITERED END SECTION	430-021	-	27.0	-	-	3 COUNT
S-44	MITERED END SECTION	430-021	-	27.0	-	-	3 COUNT
S-45	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-46	MITERED END SECTION	430-021	-	27.5	-	-	3 COUNT
S-47	MITERED END SECTION	430-021	27.0	-	-	-	3 COUNT
S-48	MITERED END SECTION	430-021	27.0	-	-	-	3 COUNT
S-49	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-50	MITERED END SECTION	430-021	27.5	-	-	-	3 COUNT
S-51	MITERED END SECTION	430-021	27.5	-	-	-	1 COUNT
S-52	MITERED END SECTION	430-021	-	-	27.5	-	1 COUNT
S-53	MITERED END SECTION	430-021	-	27.5	-	-	1 COUNT

MATCH LINE (REFER TO E-MAR02 SHEET 6 OF 11)

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM D. HEIKKINEN, P.E. ON AUGUST 18, 2026.
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PERMIT DRAWINGS
DATE: AUGUST 18, 2026

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SOMI ENGINEERING, LLC
2332 GALIANO STREET 2ND FLOOR, CORAL GABLES, FL 33134
P: (561) 901-9255
FLORIDA CERTIFICATE OF AUTHORIZATION NO. 32647

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026 SCALE: AS NOTED

POWER DELIVERY

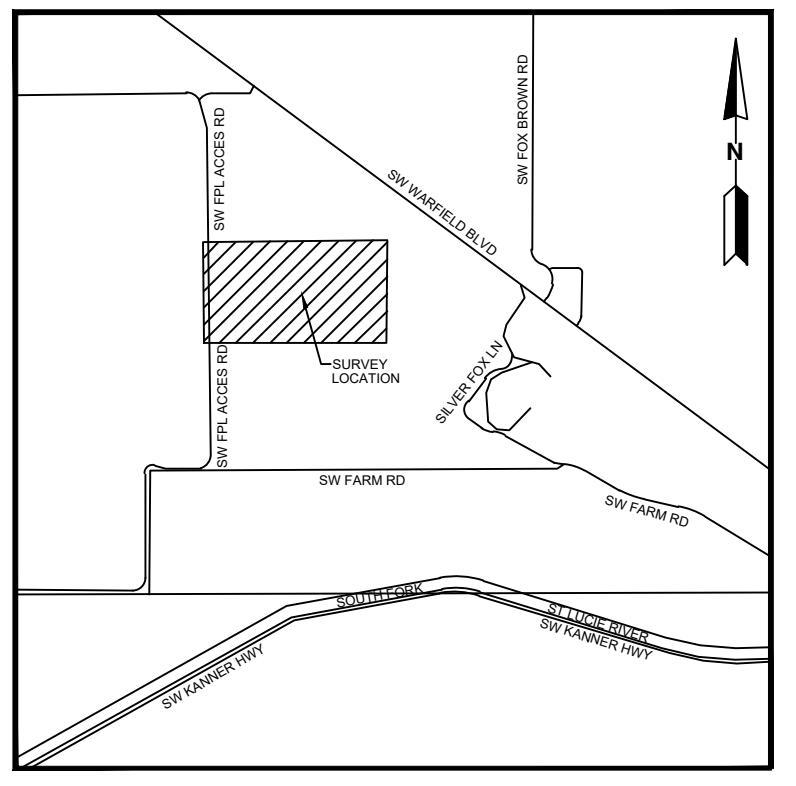
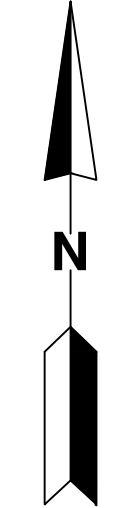
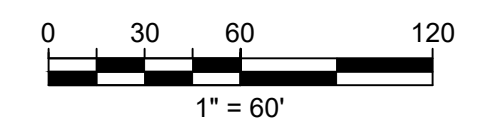
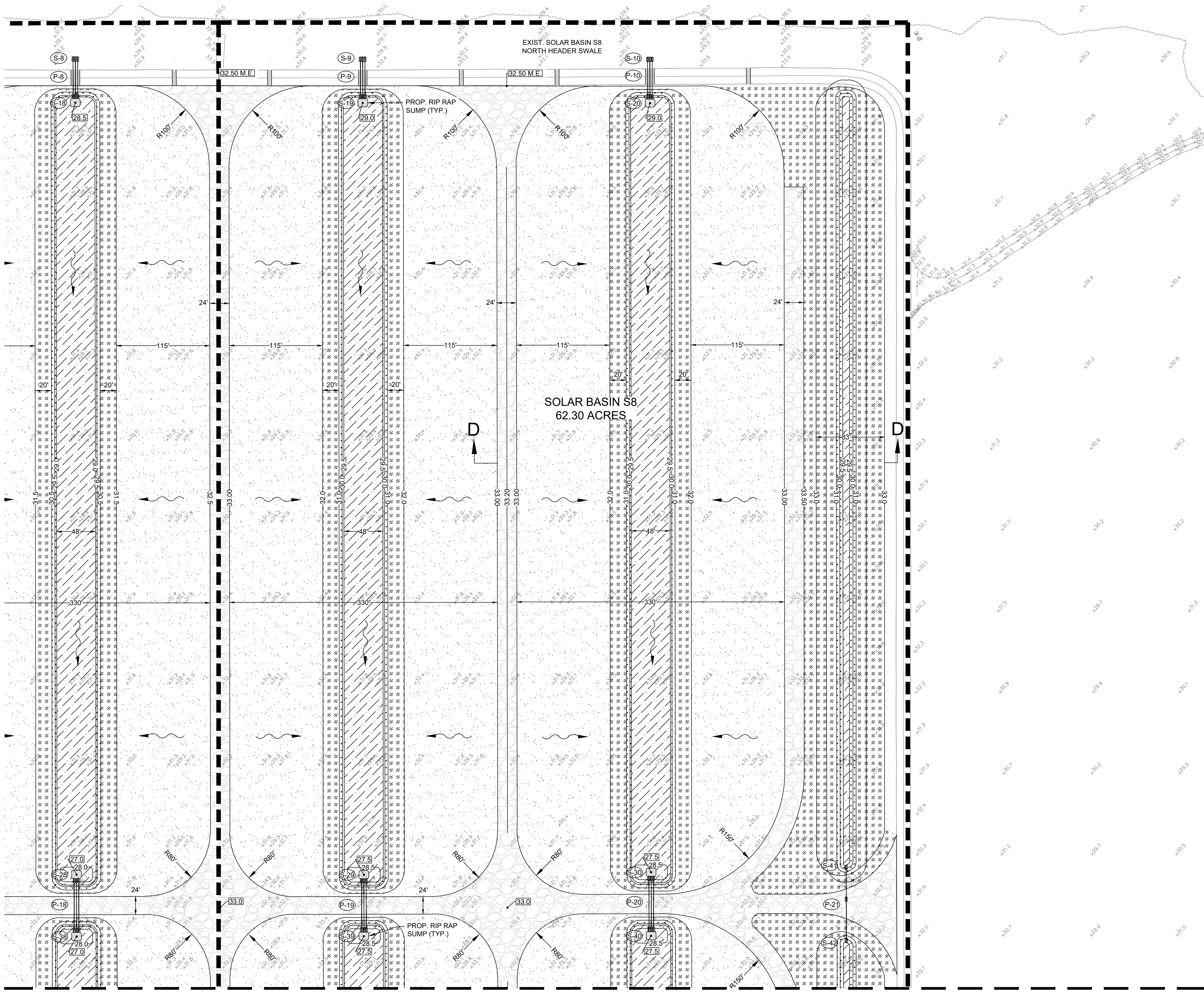
DR: N/A ER: WDH
CH: WDH COR: N/A
APPROVED:

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE PLAN

SOMI NO. 26008 FPL NO.
SHEET 3 OF 11
E-MAR02

REV NO.

WILLIAM D. HEIKKINEN
No 78529
STATE OF FLORIDA
PROFESSIONAL ENGINEER
WILLIAM D. HEIKKINEN
FLORIDA LICENSE NO. 78529



LOCATION SKETCH
SECTION 28/29, TOWNSHIP 39 SOUTH, RANGE 38 EAST
MARTIN COUNTY, FLORIDA
(NOT TO SCALE)

LEGEND:

- PROJECT LIMITS/BASIN BOUNDARY
- EXISTING ELEVATION, FT NAVD
- PROPOSED CONTOUR, FT NAVD
- PROPOSED COMPACTED NATIVE MATERIAL (LAYDOWN AREA)
- PROPOSED BOTTOM OF DRY RETENTION SWALE (SEED AND MULCH)
- PROPOSED SOD COVER (DRY RETENTION SWALE SLOPES ONLY)
- PROPOSED SEED AND MULCH
- PROPOSED STABILIZED ACCESS
- PROPOSED ASPHALT
 - 3" TYPE SP STRUCTURAL COURSE ASPHALT
 - 9" LIMEROCK BASE COMPACTED TO 98%, LBR 100
 - 12" SUB-BASE COMPACTED TO 98%, LBR 40 MIN.
- PROPOSED ELEVATION, FT NAVD
- PROPOSED MATCH EXISTING ELEVATION, FT NAVD
- PROPOSED DRAINAGE STRUCTURE/PIPE CALLOUTS

SECTION ID
*REFER TO E-MAR02
(SHEET 8 OF 11)

DRAINAGE PIPE SCHEDULE:

PIPE NO.	PIPE SIZE	PIPE MATERIAL	SOLID PIPE	NOTES
P-1	18"Ø	RCP	54'	3 COUNT
P-2	18"Ø	RCP	54'	3 COUNT
P-3	18"Ø	RCP	54'	3 COUNT
P-4	18"Ø	RCP	54'	3 COUNT
P-5	18"Ø	RCP	54'	3 COUNT
P-6	18"Ø	RCP	54'	3 COUNT
P-7	18"Ø	RCP	54'	3 COUNT
P-8	18"Ø	RCP	54'	3 COUNT
P-9	18"Ø	RCP	54'	3 COUNT
P-10	18"Ø	RCP	54'	3 COUNT
P-11	18"Ø	RCP	72'	3 COUNT
P-12	18"Ø	RCP	72'	3 COUNT
P-13	18"Ø	RCP	72'	3 COUNT
P-14	18"Ø	RCP	72'	3 COUNT
P-15	18"Ø	RCP	72'	3 COUNT
P-16	18"Ø	RCP	72'	3 COUNT
P-17	18"Ø	RCP	72'	3 COUNT
P-18	18"Ø	RCP	72'	3 COUNT
P-19	18"Ø	RCP	72'	3 COUNT
P-20	18"Ø	RCP	72'	3 COUNT
P-21	24"Ø	RCP	142'	1 COUNT
P-22	24"Ø	RCP	142'	3 COUNT
P-23	18"Ø	RCP	142'	3 COUNT
P-24	24"Ø	RCP	142'	3 COUNT
P-25	24"Ø	RCP	142'	3 COUNT
P-26	24"Ø	RCP	18'	1 COUNT
P-27	24"Ø	RCP	120'	1 COUNT

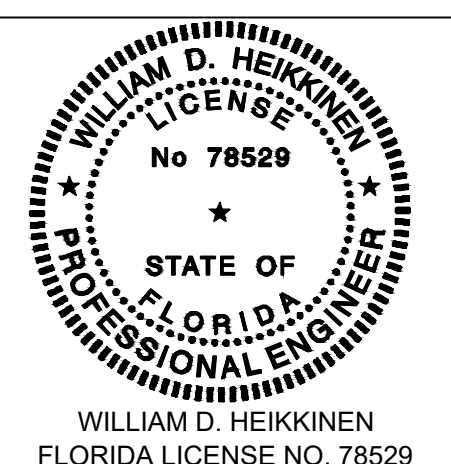
- RCP DRAINAGE PIPE JOINTS SHALL BE WRAPPED IN FILTER FABRIC (MIRAFI 160N OR APPROVED EQUAL).
- COMPACTION AROUND DRAINAGE PIPE AND STRUCTURES SHALL BE DOCUMENTED AND PROVIDED TO FPL/OWNERS REP.
- PIPE MEASUREMENTS FROM END OF M.E.S.

MATCH LINE (REFER TO E-MAR02 SHEET 7 OF 11)

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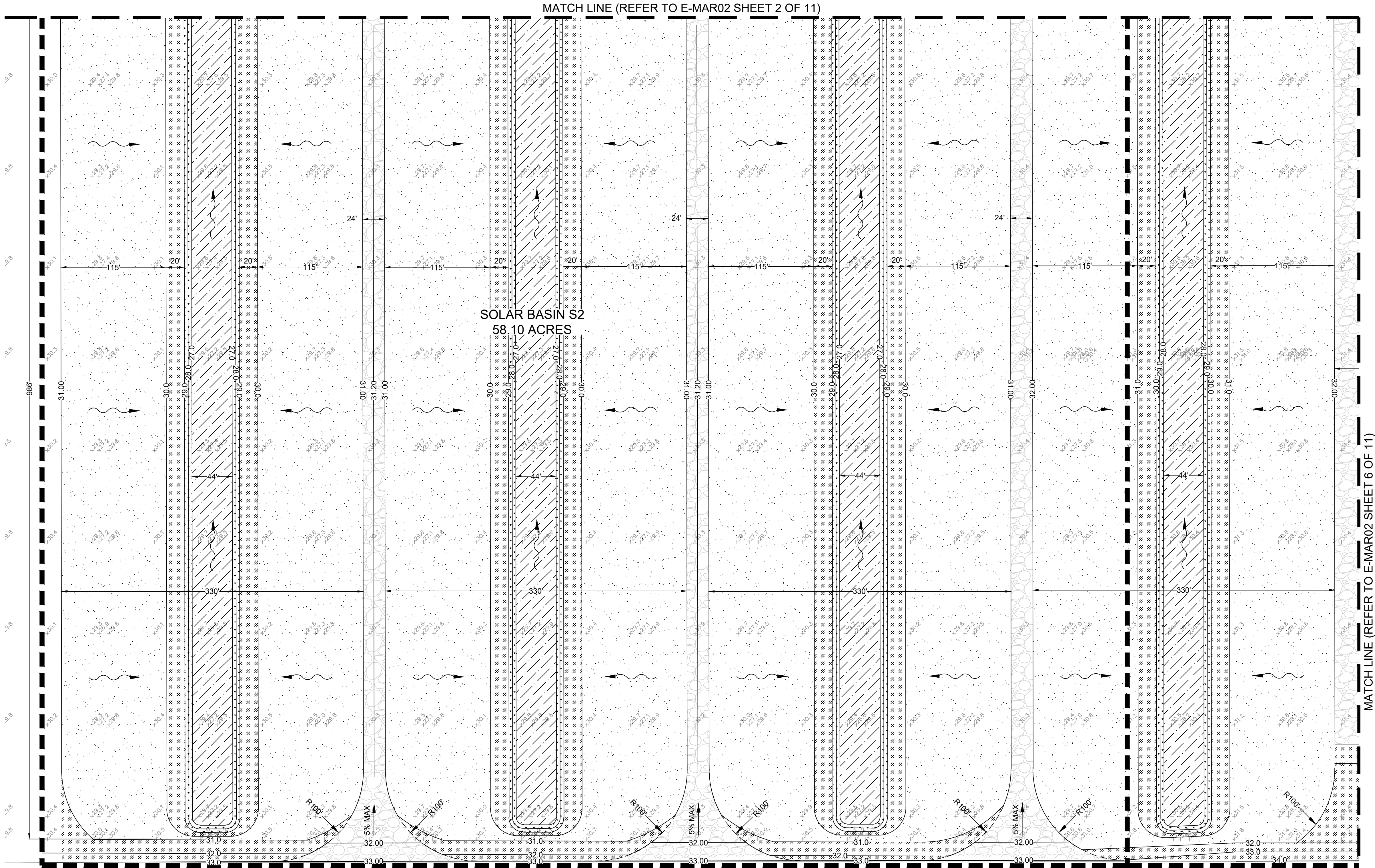
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FLORIDA CERTIFICATE OF AUTHORIZATION NO. 32647

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026	SCALE: AS NOTED
POWER DELIVERY	DR: N/A CH: WDH APPROVED:
FPL	ER: WDH COR: N/A

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE PLAN

SOMI NO. 26008
FPL NO.
SHEET 4 OF 11
E-MAR02
REV NO.



LEGEND:

- PROJECT LIMITS/BASIN BOUNDARY
- EXISTING ELEVATION, FT NAVD
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LOCATION SKETCH
SECTION 28/29, TOWNSHIP 39 SOUTH, RANGE 38 EAST
MARTIN COUNTY, FLORIDA
(NOT TO SCALE)

SECTION ID
*REFER TO E-MAR02
(SHEET 8 OF 11)

SOLAR BASIN S5
(TO REMAIN UNDISTURBED)

SOMI ENGINEERING, LLC
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APPROVED:

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE PLAN

PERMIT DRAWINGS
DATE: AUGUST 18, 2026

SOMI NO. 26008 FPL NO.

SHEET 5 OF 11
E-MAR02

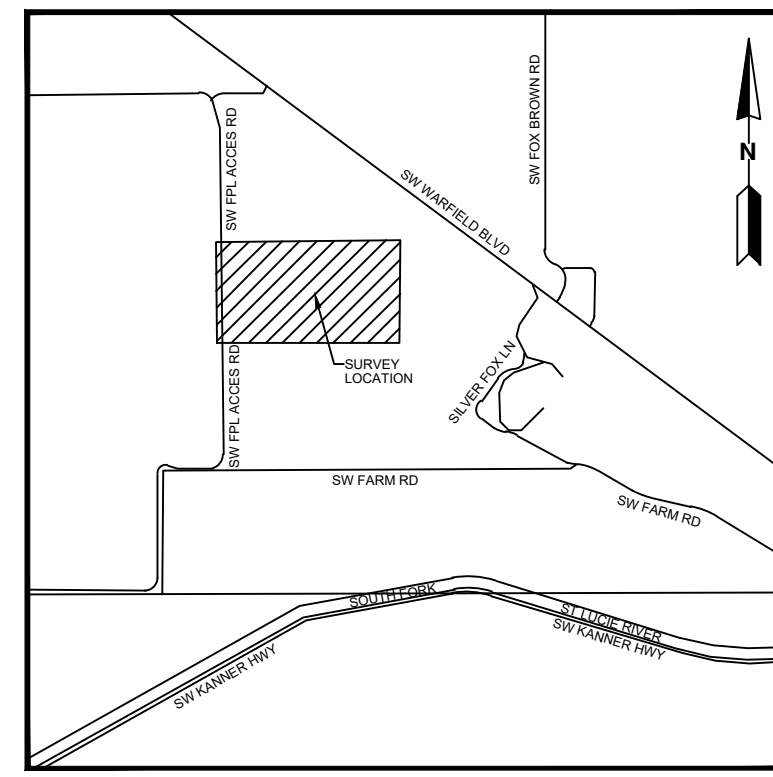
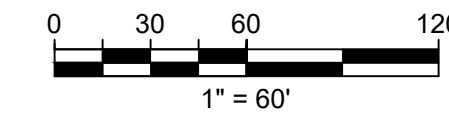
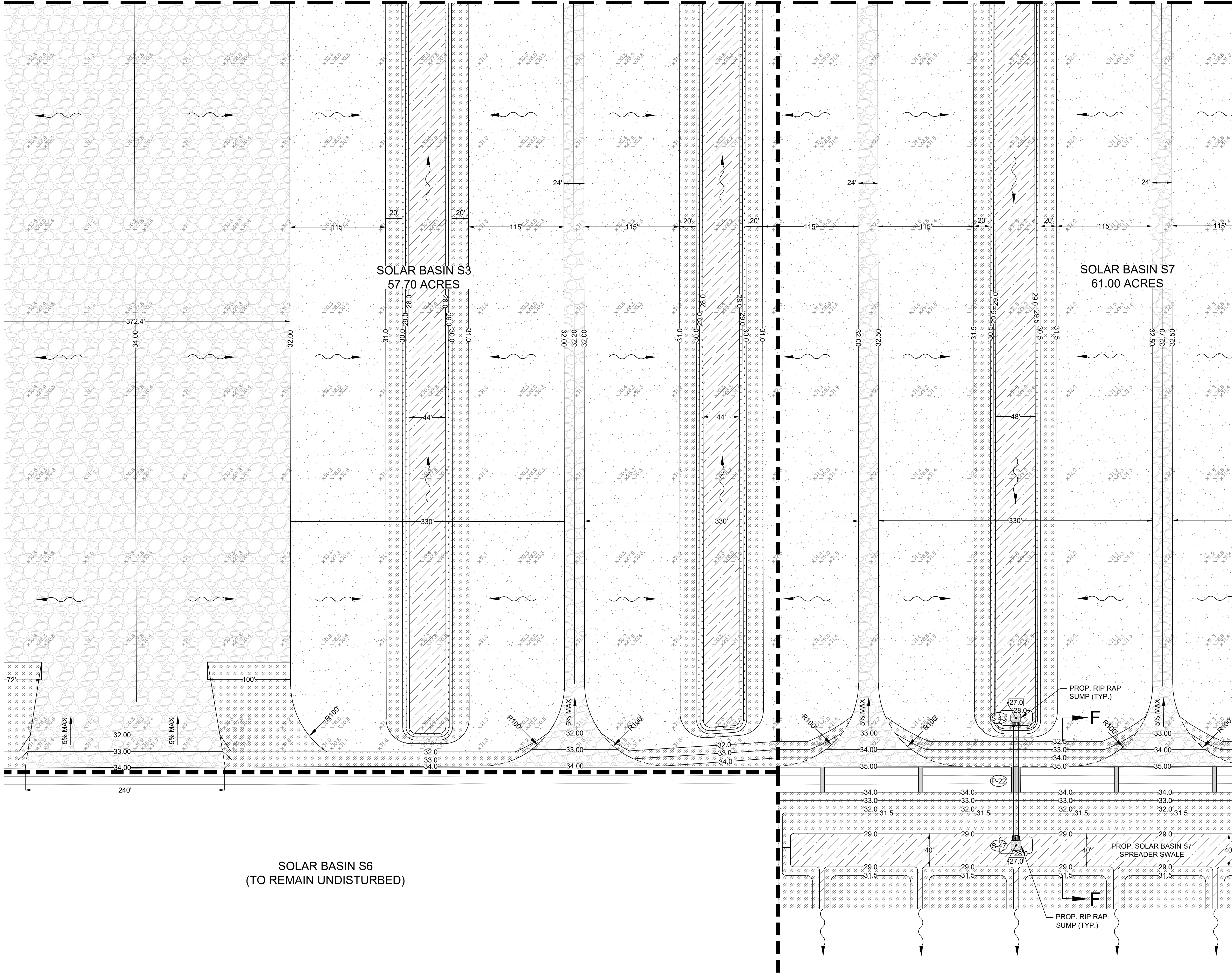
REV NO.

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No 78529
STATE OF FLORIDA
WILLIAM D. HEIKKINEN
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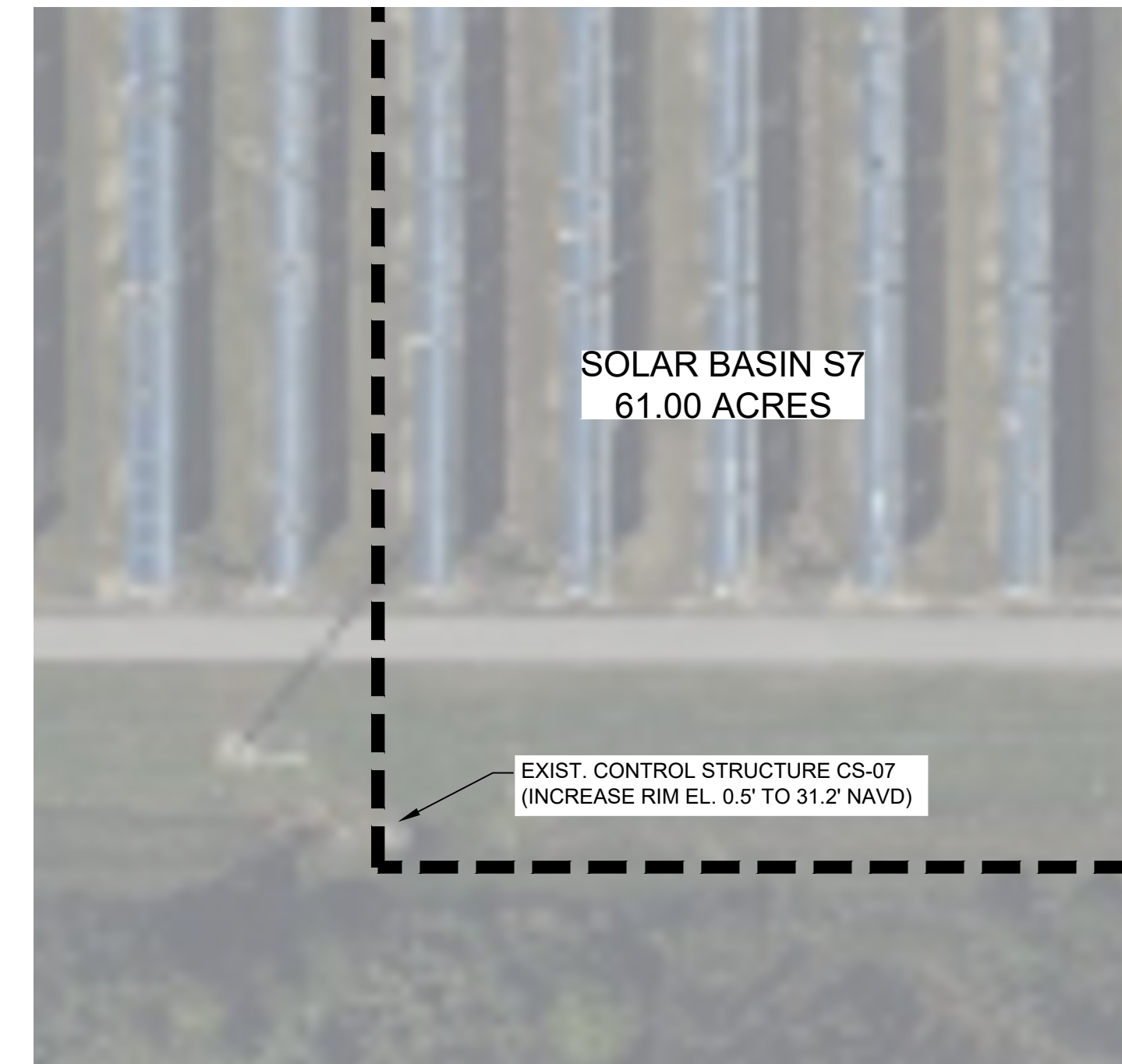
MATCH LINE (REFER TO E-MAR02 SHEET 3 OF 11)



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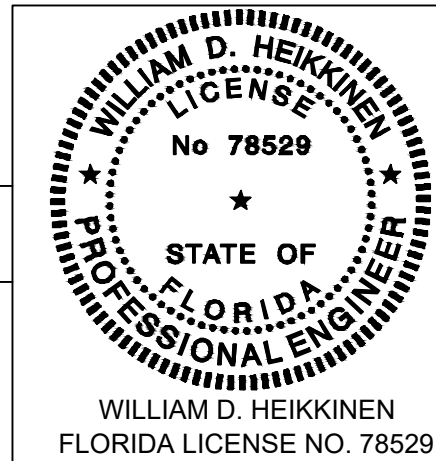
A A
SECTION ID
*REFER TO E-MAR02
(SHEET 8 OF 11)



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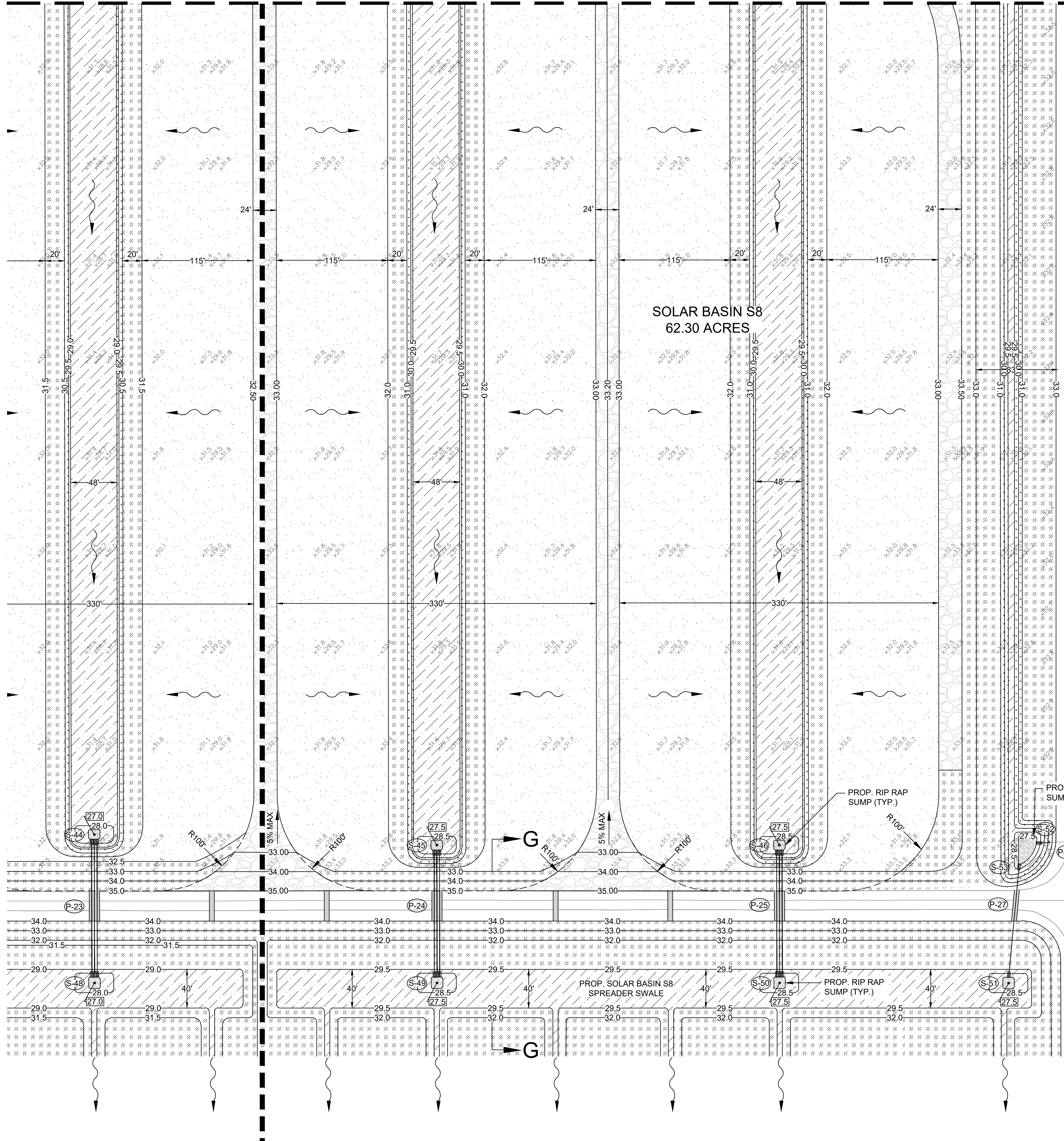
DATE: AUGUST 2026	SCALE: AS NOTED
POWER DELIVERY	DR: N/A CH: WDH APPROVED:
FPL	ER: WDH COR: N/A

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE PLAN

SOMI NO. 26008
FPL NO.
SHEET 6 OF 11
E-MAR02

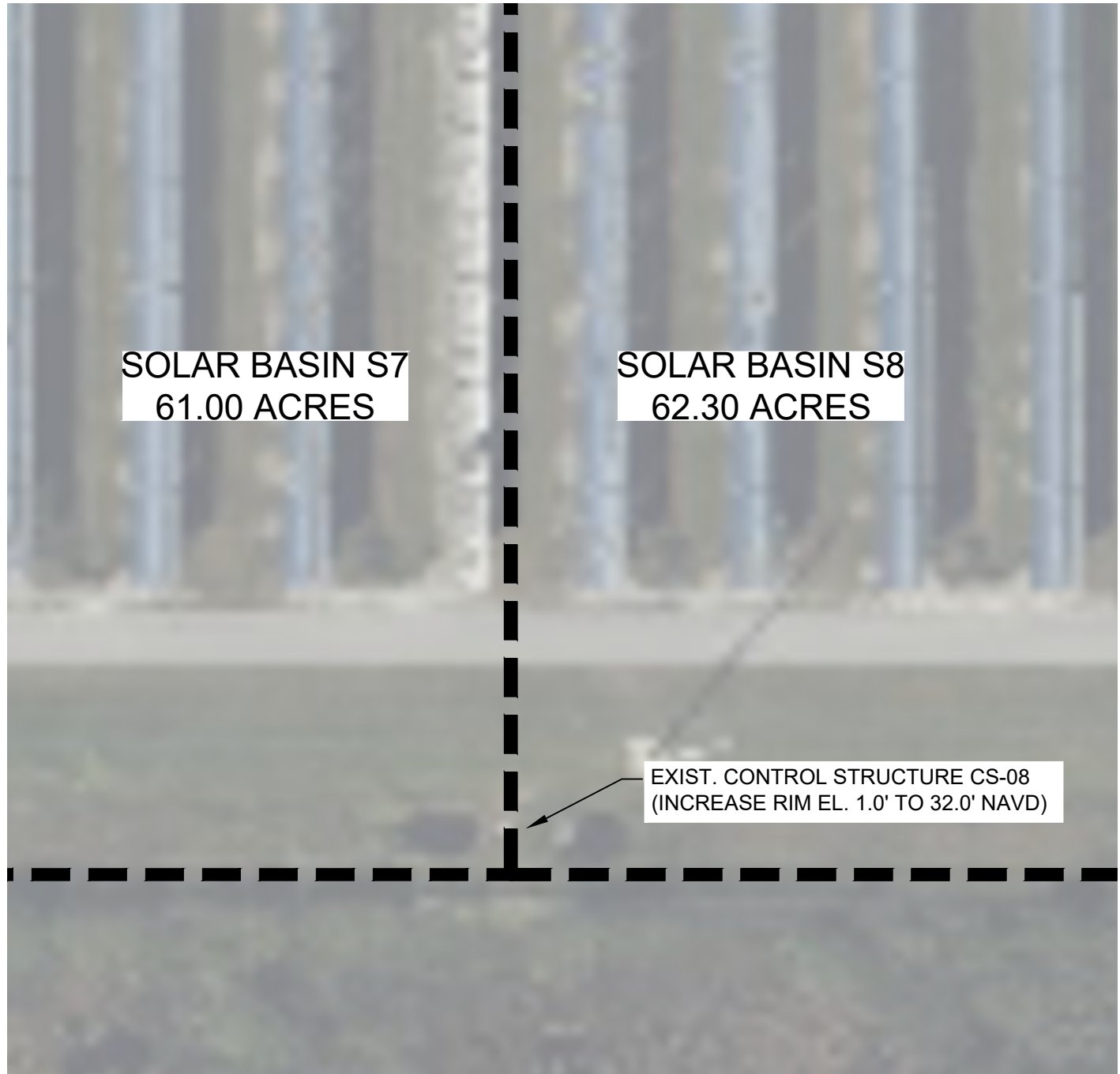
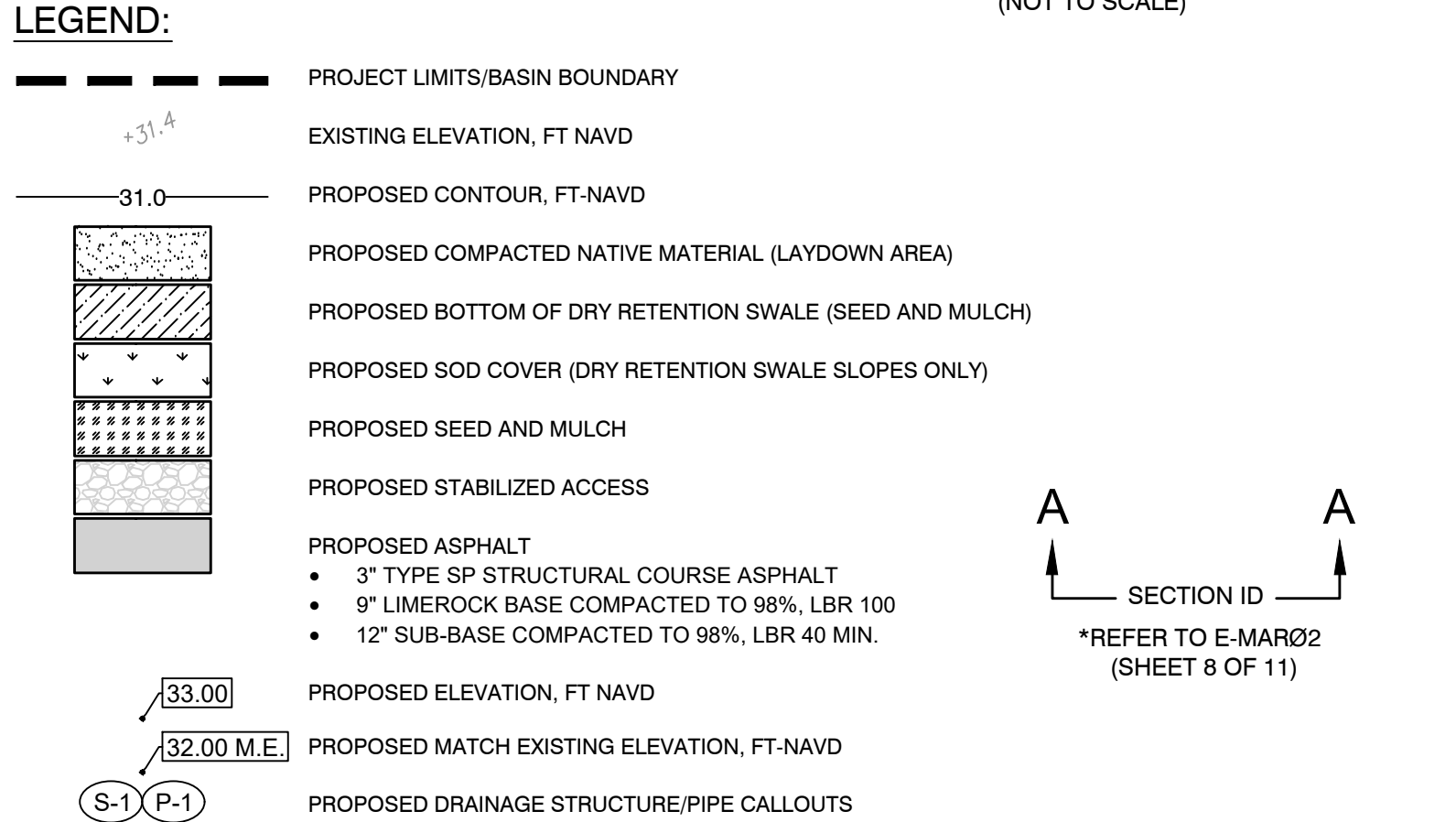
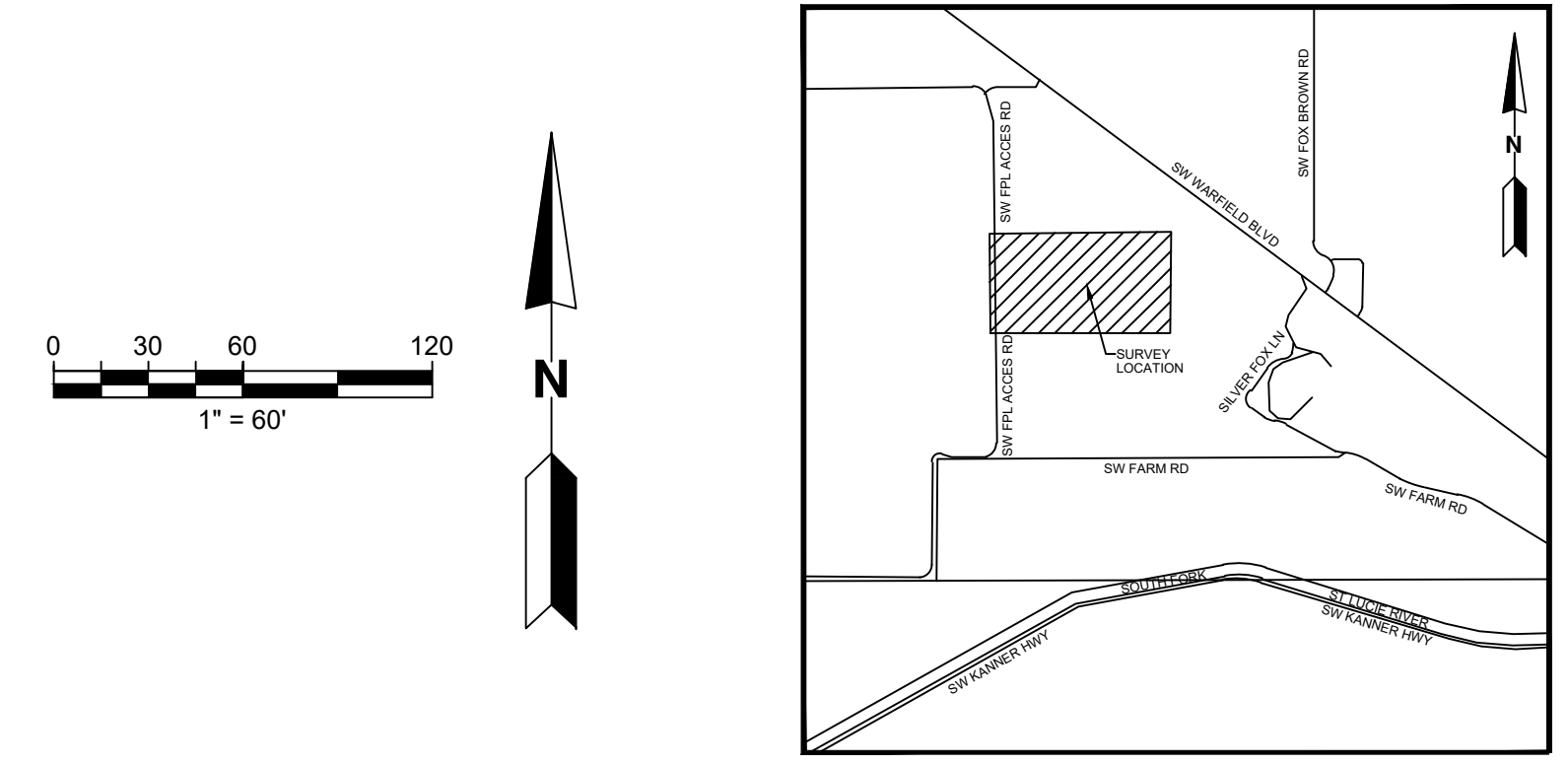
REV NO.

MATCH LINE (REFER TO E-MAR02 SHEET 4 OF 11)



SOLAR BASIN 7
(TO REMAIN UNDISTURBED)

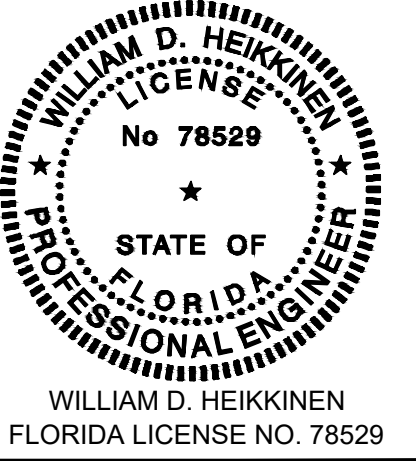
EXIST. WET
DETENTION



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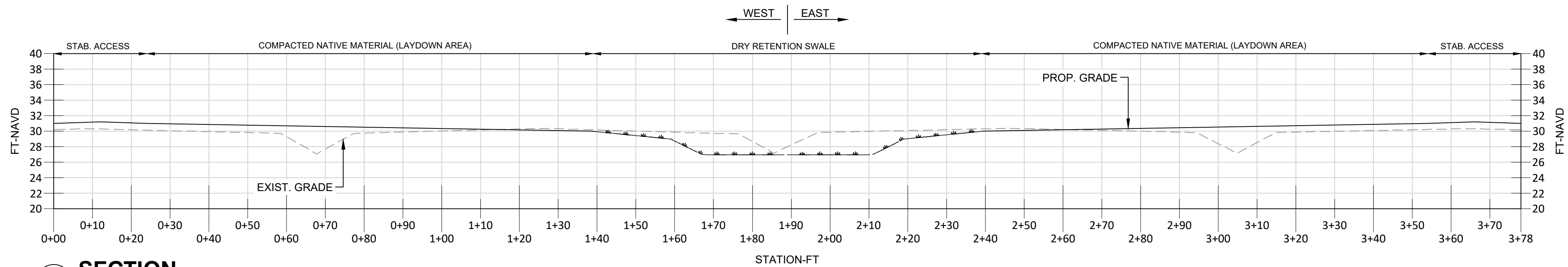
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		APPROVED:	

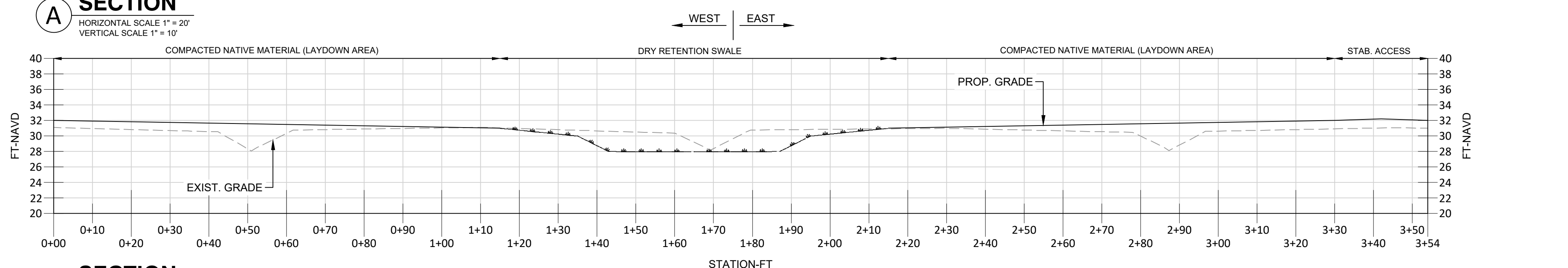
MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE PLAN

SOMI NO. 26008
FPL NO.
SHEET 7 OF 11
E-MAR02

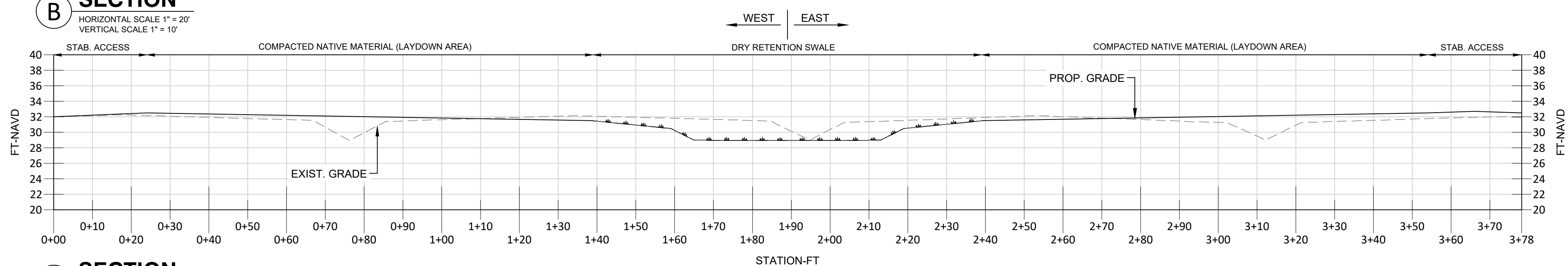
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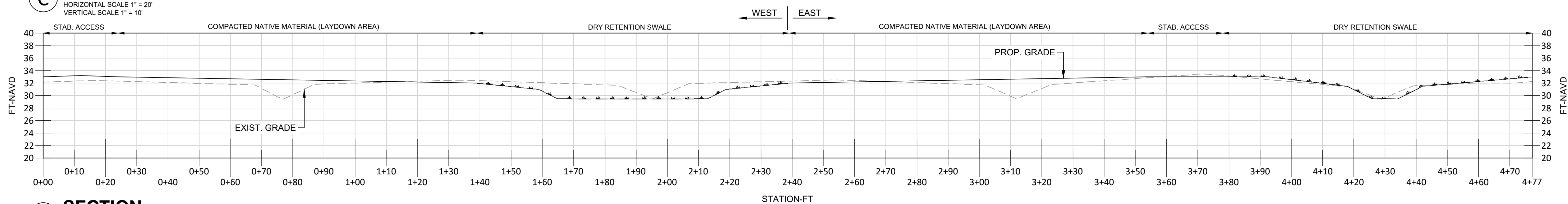
A SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



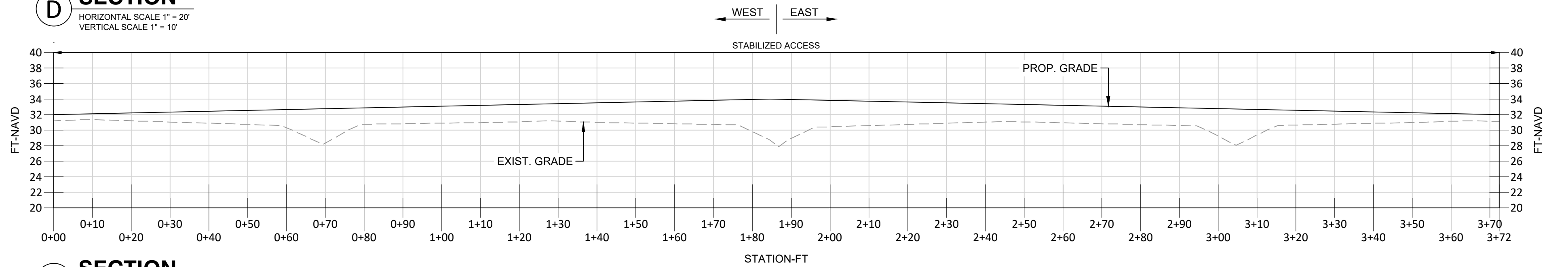
B SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



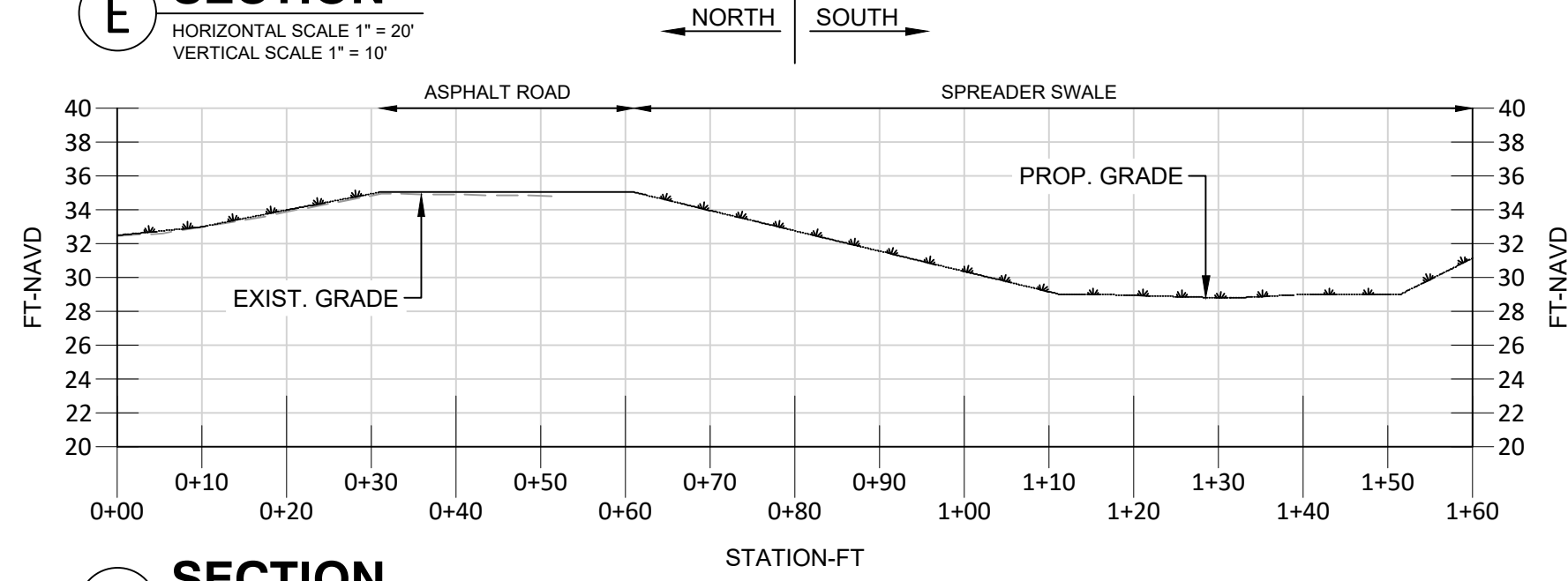
C SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



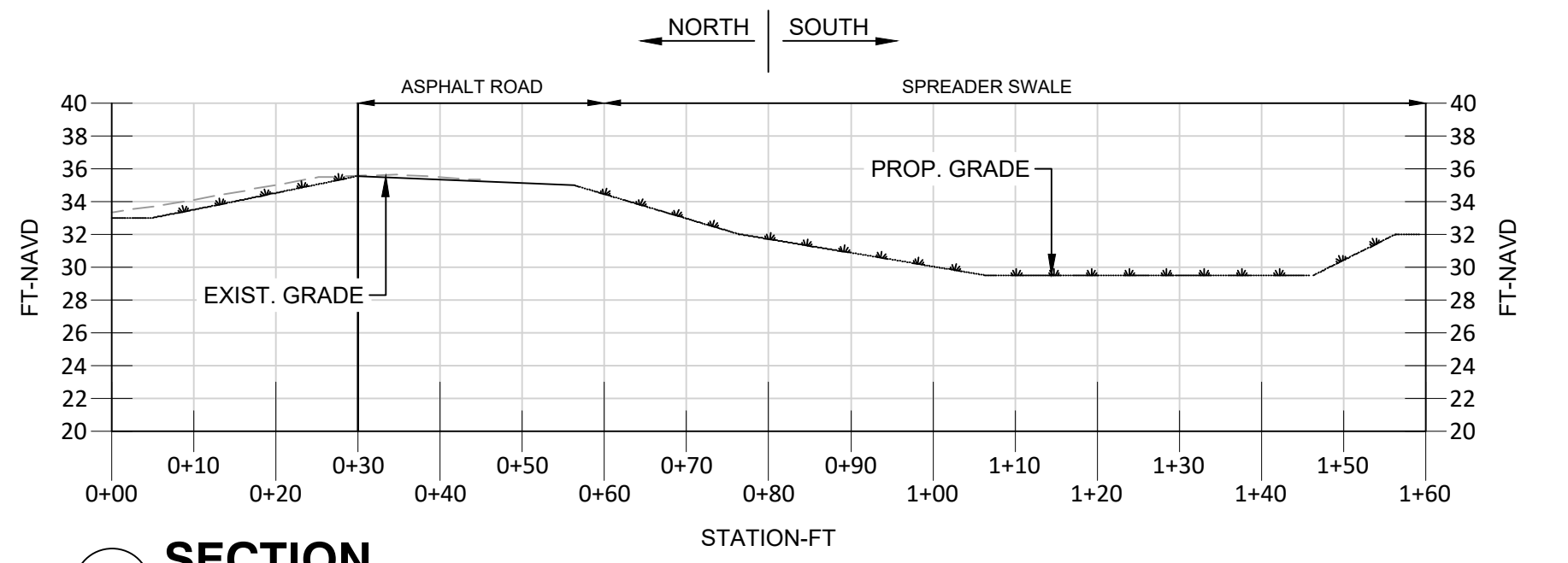
D SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



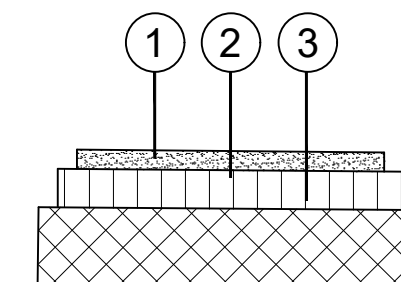
E SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



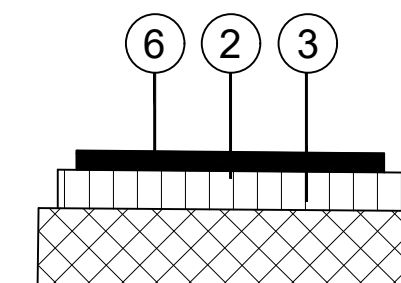
F SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



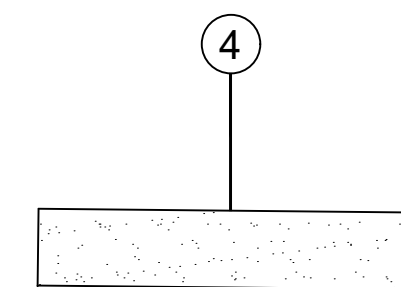
G SECTION
HORIZONTAL SCALE 1" = 20'
VERTICAL SCALE 1" = 10'



TYPICAL STABILIZED ACCESS SECTION



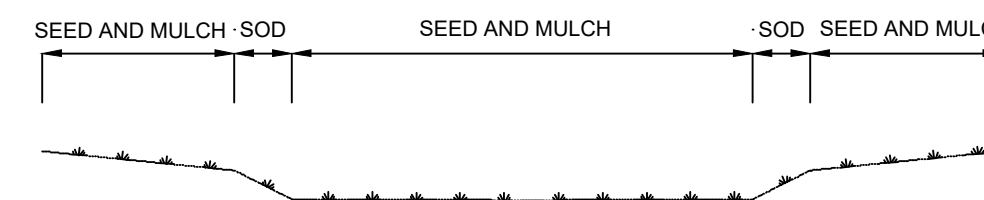
TYPICAL ASPHALT PAVEMENT REPAIR SECTION



TYPICAL LAYDOWN YARD SECTION

LEGEND

- ① 2" #5 LOOSE ROCK COVER (DUST FREE)
- ② 9" LIMEROCK BASE COMPACTED TO 98%, LBR 100
- ③ 12" LIMEROCK SUB-BASE COMPACTED TO 98%, LBR 40 MIN.
- ④ COMPACTED NATIVE MATERIAL
- ⑤ SOD COVER/SEED AND MULCH
- ⑥ ASPHALTIC CONCRETE SURFACE COURSE 3" TO BE PLACED IN TWO LIFTS (2" SP-12.5 AND 1" FC-9.5)



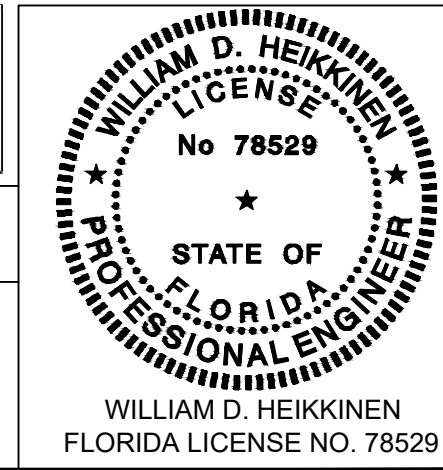
DRY RETENTION SWALE GROUND COVER

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MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
FENCE FILL AND GRADE SECTIONS

SOMI NO. 26008
FPL NO.
SHEET 8 OF 11
E-MAR02

REV NO.

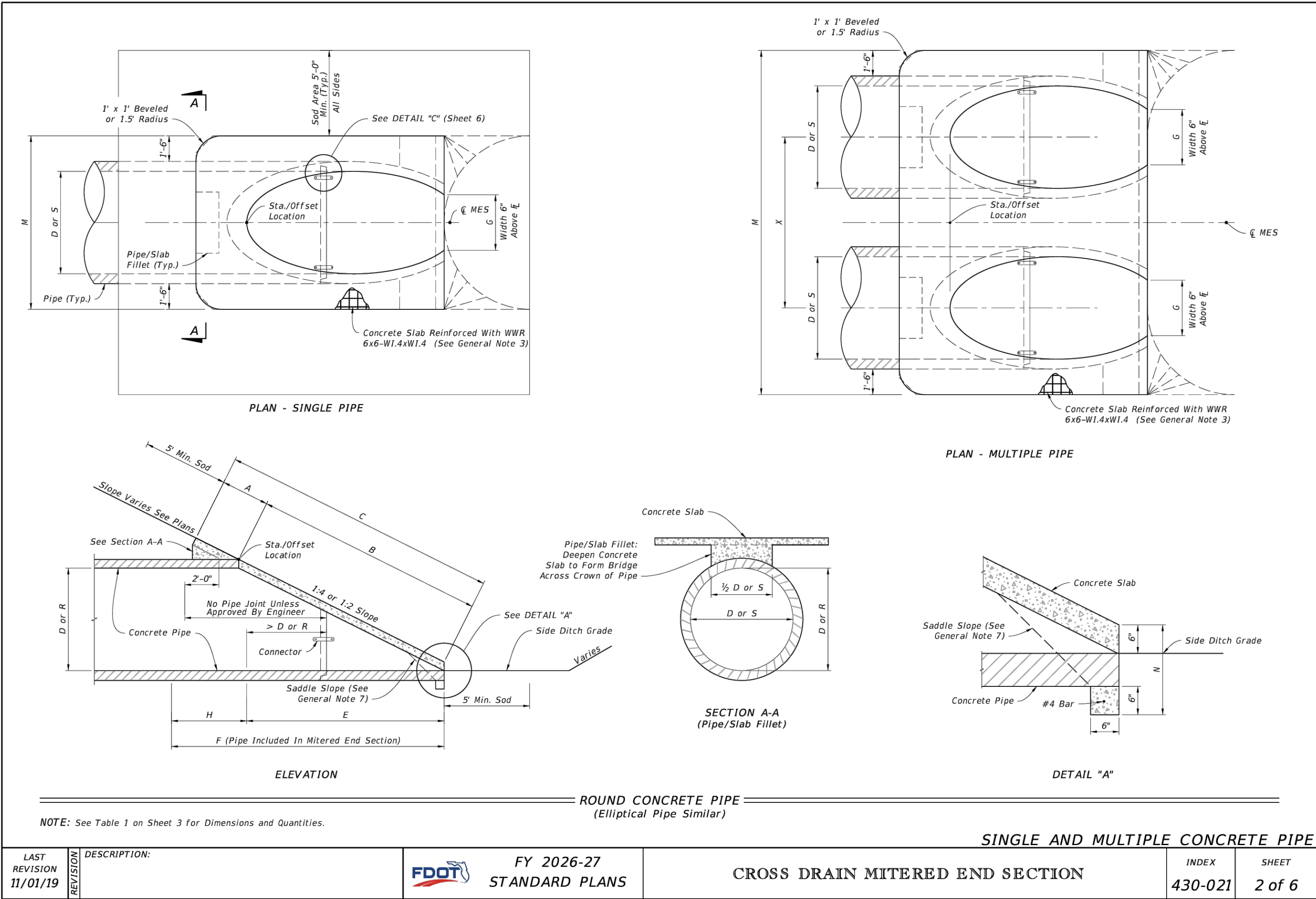


TABLE 1																											
SINGLE AND MULTIPLE CONCRETE PIPE DIMENSIONS AND QUANTITIES																											
Dia. D	Rise R	Span S	X	A	B	C	E	F	G	H	M				N	3/4" CONC. SLAB (CY) (See General Note 3)				3" CONC. SLAB (CY) (See General Note 3)				SODDING (SY)			
											Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe		Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe	Single Pipe	Double Pipe	Triple Pipe	Quad. Pipe
15"	---	2'-7"	1.92	2.18	4.10	2.06	5	1.22	2.9	4.63	7.21	9.79	12.37	1.19	0.38	0.58	0.77	0.96	0.27	0.41	0.54	0.67	21	24	27	30	
18"	---	2'-10"	1.97	2.74	4.71	2.56	6	1.41	3.4	4.92	7.75	10.58	13.42	1.21	0.44	0.65	0.87	1.09	0.31	0.45	0.60	0.75	22	25	28	31	
24"	---	3'-5"	2.06	3.85	5.91	3.36	7	1.73	3.4	5.50	8.92	12.33	15.75	1.25	0.54	0.83	1.12	1.42	0.39	0.59	0.79	1.00	24	28	32	35	
30"	---	4'-3"	2.15	4.95	7.10	4.56	8	2.00	3.4	6.08	10.33	14.58	18.83	1.29	0.66	1.09	1.50	1.91	0.46	0.76	1.04	1.32	26	31	35	40	
36"	---	5'-1"	2.25	6.08	8.37	5.56	9	2.24	3.4	6.67	11.75	16.83	21.92	1.33	0.81	1.38	1.95	2.51	0.55	0.94	1.33	1.71	28	34	39	45	
42"	---	6'-0"	2.34	7.21	9.55	6.56	10	2.45	3.4	7.25	13.25	19.25	25.25	1.38	0.97	1.70	2.45	3.19	0.66	1.15	1.66	2.15	30	37	43	50	
48"	---	6'-9"	2.43	8.33	10.76	7.56	11	2.65	3.4	7.83	14.58	21.33	28.08	1.42	1.13	2.04	2.93	3.84	0.76	1.37	1.96	2.57	32	39	47	54	
54"	---	7'-8"	2.52	9.44	11.96	8.56	12	2.83	3.4	8.42	16.08	23.75	31.42	1.46	1.31	2.44	3.58	4.72	0.87	1.62	2.38	3.14	34	42	51	59	
60"	---	8'-6"	2.62	10.56	13.18	9.56	14	3.00	4.4	9.00	17.50	26.00	34.50	1.50	1.51	2.89	4.28	5.68	0.99	1.90	2.81	3.73	36	45	55	64	
66"	---	9'-2"	2.71	11.68	14.39	10.56	15	3.18	4.4	9.58	18.75	27.92	37.08	1.54	1.68	3.25	4.84	6.43	1.11	2.15	3.21	4.27	38	48	58	68	
72"	---	10'-0"	2.80	12.80	15.60	11.56	16	3.36	4.4	10.16	20.16	30.16	40.16	1.58	1.89	3.74	5.59	7.45	1.24	2.46	3.68	4.90	40	51	62	73	
15"	---	2'-7"	2.27	4.09	6.36	4.03	6	1.25	4.0	4.63	7.21	9.79	12.37	1.19	0.57	0.87	1.15	1.44	0.40	0.61	0.80	1.00	23	26	29	32	
18"	---	2'-10"	2.36	5.12	7.48	5.03	7	1.41	4.0	4.92	7.75	10.58	13.42	1.21	0.66	0.99	1.31	1.65	0.47	0.69	0.91	1.14	25	28	31	35	
24"	---	3'-5"	2.53	7.18	9.71	7.03	8	1.73	4.0	5.50	8.92	12.33	15.75	1.25	0.85	1.30	1.75	2.20	0.60	0.90	1.21	1.52	28	32	36	40	
30"	---	4'-3"	2.70	8.25	11.55	8.03	10	2.00	4.0	6.08	10.33	14.58	18.83	1.29	1.10	1.74	2.38	3.05	0.76	1.19	1.63	2.07	31	36	41	46	
36"	---	5'-1"	2.87	11.31	14.18	11.03	12	2.24	4.0	6.67	11.75	16.83	21.92	1.33	1.32	2.21	3.08	3.96	0.89	1.48	2.05	2.63	34	40	46	52	
42"	---	6'-0"	3.05	13.37	16.42	13.03	14	2.45	4.0	7.25	13.25	19.25	25.25	1.38	1.58	2.76	3.91	5.09	1.05	1.82	2.57	3.34	38	44	51	58	
48"	---	6'-9"	3.22	15.43	18.65	15.03	16	2.65	4.0	7.83	14.58	21.33	28.08	1.42	1.85	3.30	4.73	6.17	1.21	2.15	3.07	4.00	41	48	56	63	
54"	---	7'-8"	3.39	17.49	20.88	17.03	18	2.83	4.0	8.42	16.08	23.75	31.42	1.46	2.14	3.95	5.77	7.58	1.39	2.55	3.72	4.88	44	52	61	69	
60"	---	8'-6"	3.56	19.55	23.11	19.03	20	3.00	4.0	9.00	17.50	26.00	34.50	1.50	2.45	4.66	6.87	9.07	1.59	3.02	4.44	5.86	47	56	66	75	
66"	---	9'-2"	3.73	21.62	25.55	21.03	22	3.18	4.0	9.58	18.75	27.92	37.08	1.54	2.88	5.54	8.18	10.84	1.91	3.66	5.40	7.15	49	59	69	80	
72"	---	10'-0"	3.91	23.68	27.59	23.03	24	3.36	4.0	10.16	20.16	30.16	40.16	1.58	3.54	6.61	9.87	13.13	2.12	4.18	6.24	8.30	52	63	74	85	
12"	18"	2'-0"	1.97	1.62	3.59	1.56	4	1.50	2.4	4.92	7.75	10.58	13.42	1.21	0.30	0.49	0.67	0.85	0.19	0.33	0.45	0.57	21	24	27	30	
14"	23"	3'-4"	2.01	1.99	4.00	1.89	5	1.90	3.1	5.38	8.71	12.04	15.38	1.23	0.37	0.59	0.81	1.02	0.25	0.40	0.55	0.69	22	26	29	33	
19"	30"	4'-0"	2.11	2.92	5.03	2.73	6	2.37	3.3	6.04	10.04	14.04	18.04	1.27	0.50	0.80	1.09	1.39	0.34	0.55	0.75	0.95	24	28	33	37	
24"	38"	5'-0"	2.20	3.85	6.05	3.46	7	2.83	3.4	6.79	11.79	16.79	21.79	1.31	0.62	1.03	1.45	1.86	0.43	0.71	1.00	1.28	26	31	37	42	
29"	45"	5'-11"	2.34	4.79	7.13	4.39	8	3.19	3.6	7.50	13.42	19.33	25.25	1.38	0.75	1.30	1.84	2.39	0.52	0.90	1.27	1.65	28	34	41	47	
34"	53"	7'-0"	2.43	5.72	8.15	5.23	9	3.57	3.8	8.25	15.25	22.25	29.25	1.42	0.90	1.61	2.32	3.03	0.62	1.11	1.60	2.09	30	37	45	53	
38"	60"	7'-10"	2.52	6.46	8.98	5.89	9	3.95	3.1	8.92	16.75	24.58	32.42	1.46	1.03	1.89	2.74	3.60	0.70	1.29	1.87	2.46	31	40	49	57	
43"	68"	8'-11"	2.62	7.39	10.01	6.73	10	4.28	3.3	9.67	18.58	27.50	36.42	1.50	1.19	2.26	3.33	4.40	0.81	1.54	2.26	2.99	33	43	53	63	
48"	76"	9'-11"	2.71	8.33	11.04	7.56	11	4.59	3.4	10.42	20.33	30.25	40.17	1.54	1.38	2.65	3.93	5.21	0.93	1.79	2.66	3.53	35	46	57	68	
53"	83"	10'-8"	2.80	9.26	12.06	8.39	12	4.77	3.6	11.08	21.75	32.42	43.08	1.58	1.55	3.03	4.50	5.96	1.04	2.04	3.03	4.02	37	49	61	73	
58"	91"	11'-8"	2.90	10.19	13.09	9.23	13	5.01	3.8	11.83	23.50	35.17	46.83	1.63	1.75	3.47	5.20	6.93	1.17	2.33	3.49	4.66	39	52	65	78	
12"	18"	2'-0"	2.36	3.06	5.42	3.03	5	1.50	2.0	4.92	7.75	10.58	13.42	1.21	0.45	0.68	0.92	1.14	0.30	0.45	0.61	0.76	23	26	29	32	
16"	23"	3'-4"	2.44	3.75	6.19	3.70	6	1.90	2.3	5.38	8.71	12.04	15.38	1.23	0.53	0.83	1.13	1.42	0.36	0.56	0.76	0.95	24	28	32	35	
19"	30"	4'-0"	2.62	5.47	8.09	5.36	8	2.37	2.6	6.04	10.04	14.04	18.04	1.27	0.74	1.15	1.57	1.98	0.51	0.79	1.08	1.36	27	32	36	40	
24"	38"	5'-0"	2.79	7.18	9.97	7.03	10	2.85	3.0	6.79	11.79	16.79	21.79	1.31	0.97	1.57	2.19	2.81	0.68	1.10	1.53	1.96	30	36	41	47	
29"	45"	5'-11"	3.05	8.90	11.95	8.70	12	3.19	3.3	7.50	13.42	19.33	25.25	1.38	1.22	2.07	2.92	3.77	0.86	1.45	2.04	2.63	33	40	46	53	
34"	53"	7'-0"	3.22	10.62	13.84	10.36	13	3.57	2.6	8.25	15.25	22.25	29.25	1.42	1.48	2.62	3.77	4.92	1.02	1.81	2.60	3.39	36	44	52	59	
38"	60"	7'-10"	3.39	11.99	15.38	11.70	15	3.95	3.3	8.92	16.75	24.58	32.42	1.46	1.72	3.12	4.53	5.92	1.18	2.14	3.10	4.05	38	47	56	65	
43"	68"	8'-11"	3.56	13.71	17.27	13.36	17	4.28	3.6	9.67	18.58	27.50	36.42	1.50	2.02	3.78	5.56	7.32	1.38	2.58	3.79	4.99	41	51	61	71	
48"	76"	9'-11"	3.73	15.43	19.16	15.03	19	4.59	4.0	10.42	20.33	30.25	40.17	1.54	2.34	4.49	6.64	8.79	1.59	3.05	4.51	5.97	44	55	66	77	
53"	83"	10'-8"	3.91	17.15	21.06	16.70	20	4.77	3.3	11.08	21.75	32.42	43.08	1.58	2.66	5.17	7.66	10.16	1.80	3.50	5.19	6.88	47	59	71	83	
58"	91"	11'-8"	4.08	18.87	22.95	18.36	22	5.01	3.6	11.83	23.50	35.17	46.83	1.63	3.02	5.98	8.95	11.90	2.04	4.04	6.03	8.05	50	63	76	89	

△ 6.42 △ 6.25 Dimensions permitted to allow use of 8' standard pipe lengths.

◇ 10.40 ◇ 10.10 Dimensions permitted to allow use of 12' standard pipe lengths.

LAST REVISION
11/01/19

REVISION

DESCRIPTION:

FDOT

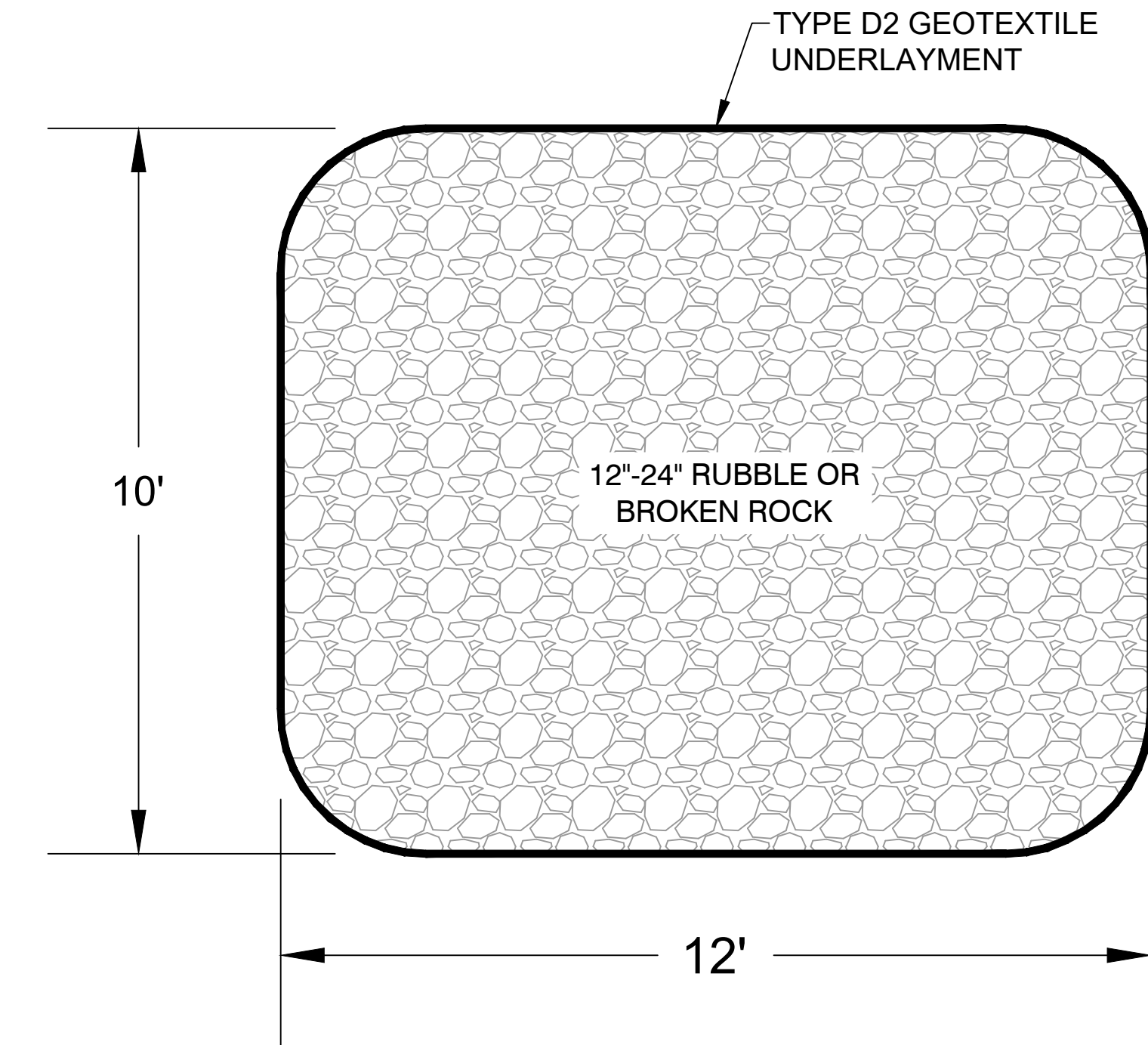
FY 2026-27
STANDARD PLANS

CROSS DRAIN MITERED END SECTION

CONCRETE PIPE DIMENSIONS AND QUANTITIES

INDEX
430-021

SHEET
3 of 6



RIP RAP SUMP DETAIL

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY WILLIAM D. HEIKKINEN, P.E. ON AUGUST 18, 2026.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

PERMIT DRAWINGS

DATE: AUGUST 18, 2026

SunshineFL

Call 811 or visit sunshinefl.com two full business days before digging to have buried facilities located and marked.

Check positive response codes before you dig!

WILLIAM D. HEIKKINEN

FLORIDA PROFESSIONAL ENGINEER

No. 78529

STATE OF FLORIDA

WILLIAM D. HEIKKINEN

FLORIDA LICENSE NO. 78529

SOMI ENGINEERING, LLC

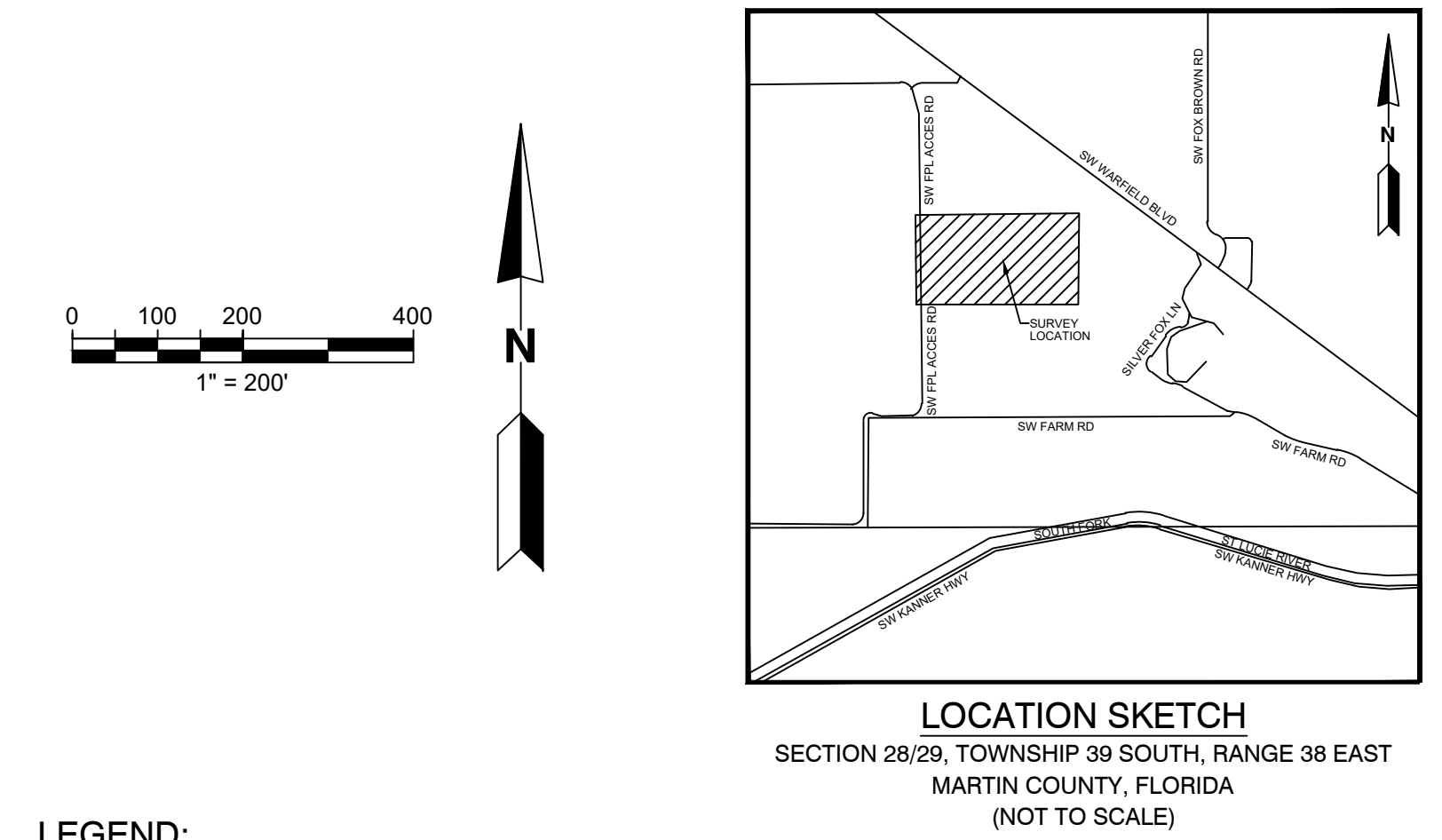
2332 GALIANO STREET 2ND FLOOR, CORAL GABLES, FL 33134

P: (661) 901-9255

FLORIDA CERTIFICATE OF AUTHORIZATION NO. 32647

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026	SCALE: AS NOTED
POWER DELIVERY	DR: N/A
	ER: WD
	CH: WD
	COR: N/A
	APPROVED:



- PROJECT LIMITS/BASIN BOUNDARY
- EXISTING ELEVATION, FT NAVD
- PROPOSED TYPE III SILT FENCE

PROJECT DESCRIPTION:
FLORIDA POWER & LIGHT CO. IS PROPOSING TO CONVERT THE FORMER MARTIN SOLAR YARD INTO A LAYDOWN YARD WITH MODIFIED DRY RETENTION SWALE WHILE MAINTAINING THE EXISTING HEADER SWALES AND CONTROL STRUCTURE.

CLEARING & GRUBBING OF EXISTING GREEN SPACE/VEGETATION, GRADING, EARTHWORK ASSOCIATED WITH SUBGRADE, BASE, LOOSE LIMESTONE COVER AND STORMWATER MANAGEMENT SYSTEM.

PROJECT AREA: ± 239.10 ACRES
TOTAL AREA TO BE DISTURBED: ± 191.45 ACRES

DRY RETENTION SWALES TO PROVIDE STORMWATER QUALITY/TREATMENT

ALL WORK SHALL TAKE PLACE WITHIN PREVIOUSLY DEVELOPED AREAS. NO WETLAND IMPACTS.

PRIOR TO THE START OF ANY SOIL DISTURBING ACTIVITIES, SILT FENCE SHALL BE INSTALLED AS SHOWN ON THE PLANS AND AS DIRECTED BY THE ENGINEER. THE SILT FENCE SHALL REMAIN IN PLACE UNTIL FINAL STABILIZATION IS ACHIEVED AND THE CONSTRUCTION IS COMPLETED OR AS DIRECTED BY THE ENGINEER. TEMPORARY CONSTRUCTION FENCE MAY BE INSTALLED AT THE CONTRACTORS DISCRETION.

STABILIZATION PRACTICES:
PERMANENT PLANTING, SODDING OR SEEDING, PAVEMENT

SILT FENCES, WIND SCREEN, BERMS, ROCK BEDDING AT CONSTRUCTION EXIT, STORM INLET SEDIMENT TRAP, FILTER FABRIC AT INLETS; CONSTRUCTION ENTRANCE/EXIT

CONTRACTOR SHALL INSTALL AND MAINTAIN THE EROSION AND SEDIMENT CONTROL MEASURES DESCRIBED AND PROVIDE ADDITIONAL MEASURES AS NECESSARY. AFTER ANY RAIN EVENT, THE CONTRACTOR WILL REVIEW ALL BMP'S AND REPAIR OR REPLACE BMP'S AS NECESSARY.

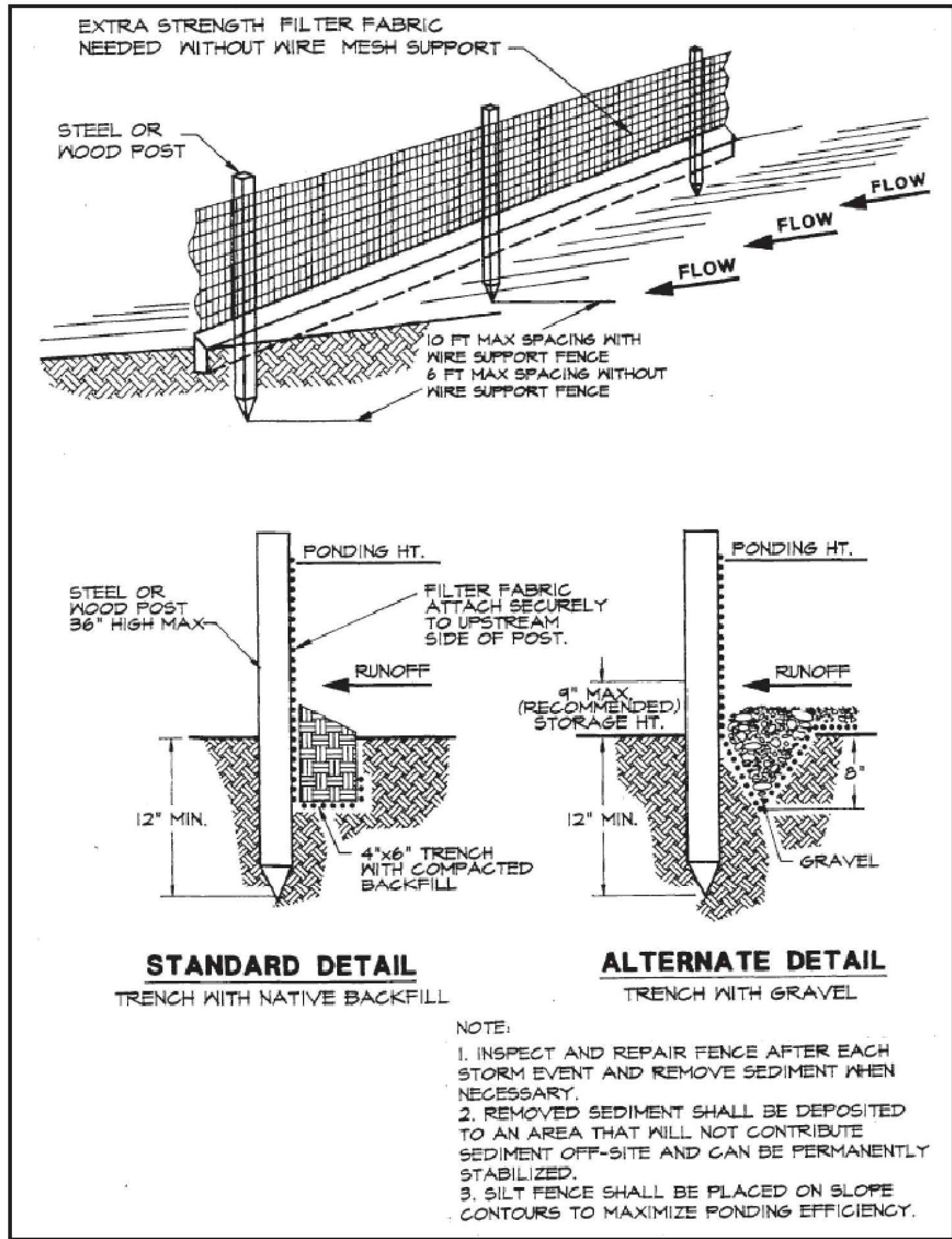
- ANY CONSTRUCTION DEBRIS GENERATED AS A RESULT OF THIS PROJECT WILL BE DISPOSED OF OFF-SITE AT AN APPROPRIATE WASTE DISPOSAL FACILITY
- CONCRETE WASH OUT LOCATIONS WILL BE PROVIDED IN AREAS WHERE THE DISPOSAL MATERIALS WILL BE CONTAINED AS TO PREVENT DISCHARGE OUTSIDE OF THE PROJECT LIMITS.
- NEW AND EXISTING DRAINAGE STRUCTURES SHALL BE PROTECTED WITH SYNTHETIC BALES AND/OR FILTER FABRIC
- STAGING AREAS WILL BE LOCATED WITHIN THE PROJECT WORK ZONE UNLESS OTHERWISE APPROVED
- A DUMPSTER MAY BE PROVIDED AT THE SITE DEPENDING ON THE AMOUNT OF LITTER GENERATED
- LOADED TRUCKS WILL BE COVERED WITH TARPS

- HAUL ROAD DAMPENED FOR DUST CONTROL
- LOADED HAUL TRUCKS TO BE COVERED WITH TARPAULIN
- STABILIZED CONSTRUCTION ENTRANCE - WITH WASH WATER CONTAINMENT WATER TRUCK
- SCHEDULING STREET SWEEPING (DAILY)

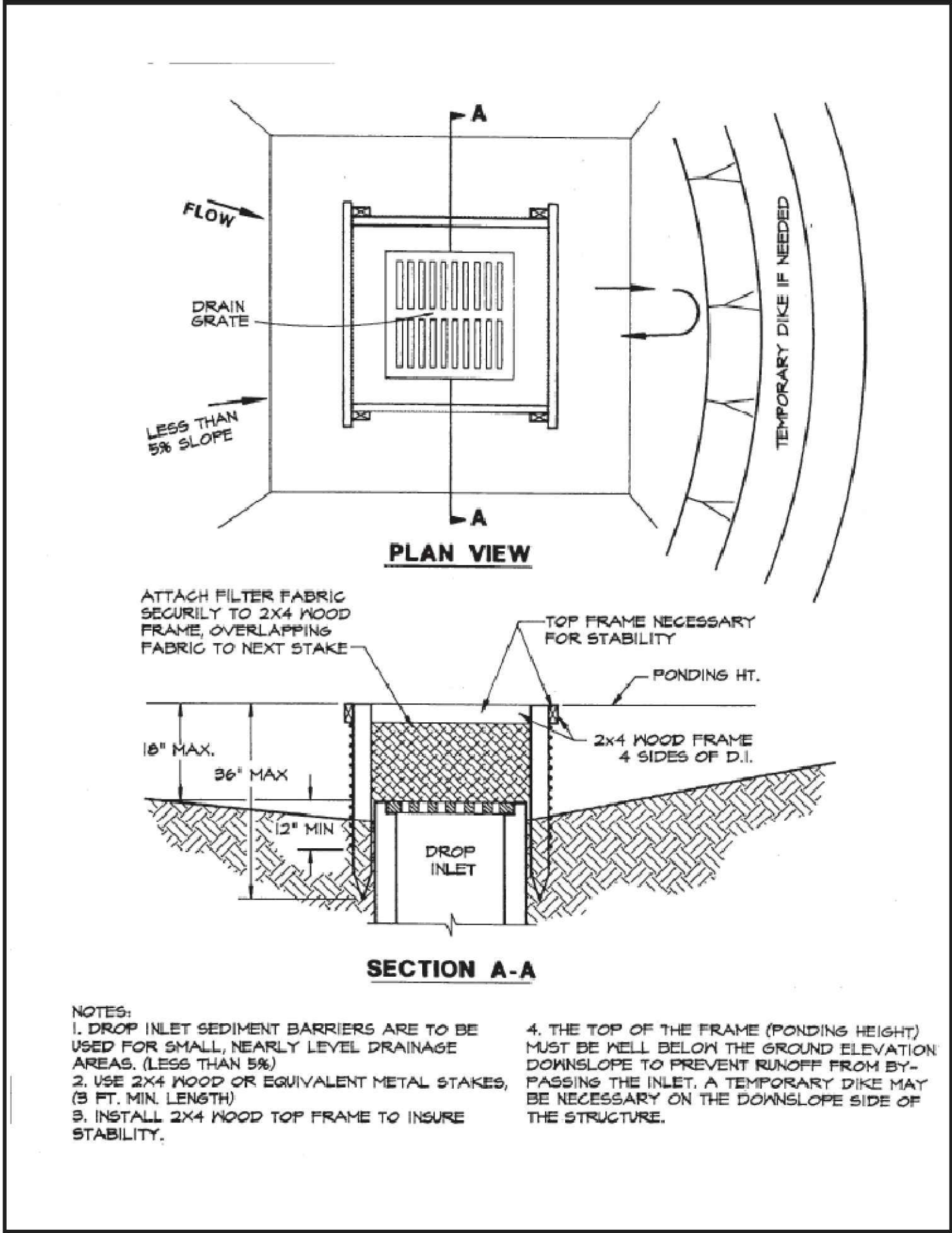
NO NON-STORMWATER DISCHARGES ARE ANTICIPATED AT THIS TIME. WHEN DEWATERING IS IMPLEMENTED, THE WATER WILL BE DEPOSITED IN A CONTROLLED AREA ON-SITE TO PREVENT OFF-SITE DISCHARGE OR AS APPROVED BY PERMIT.

LOCATIONS AND TYPES OF ALL EROSION CONTROL DEVICES SHALL BE INSTALLED AS DIRECTED BY PLAN AND/OR BY FPL. CONTRACTOR SHALL IMPLEMENT THE SWPPP AS PER CONTRACT PLANS. HOWEVER, IT MAY BE REVISED BASED ON ACTUAL FIELD CONDITIONS AT THE TIME WORK IS BEING PERFORMED. MONITORING SHALL BE PERFORMED ON A WEEKLY BASIS AND AFTER A 1/2" STORM EVENT WITH ROUTINE MAINTENANCE AND REPLACEMENT OF ANY DEVICES AS REQUIRED. CONTAINMENT OR REMOVAL OF POLLUTANTS SHALL BE IN ACCORDANCE WITH ALL APPLICABLE STATE AND/OR FDOT STANDARD SPECIFICATIONS. THE RESPONSIBLE PERSONS FOR MONITORING AND MAINTAINING THE EROSION CONTROL DEVICES SHALL BE:

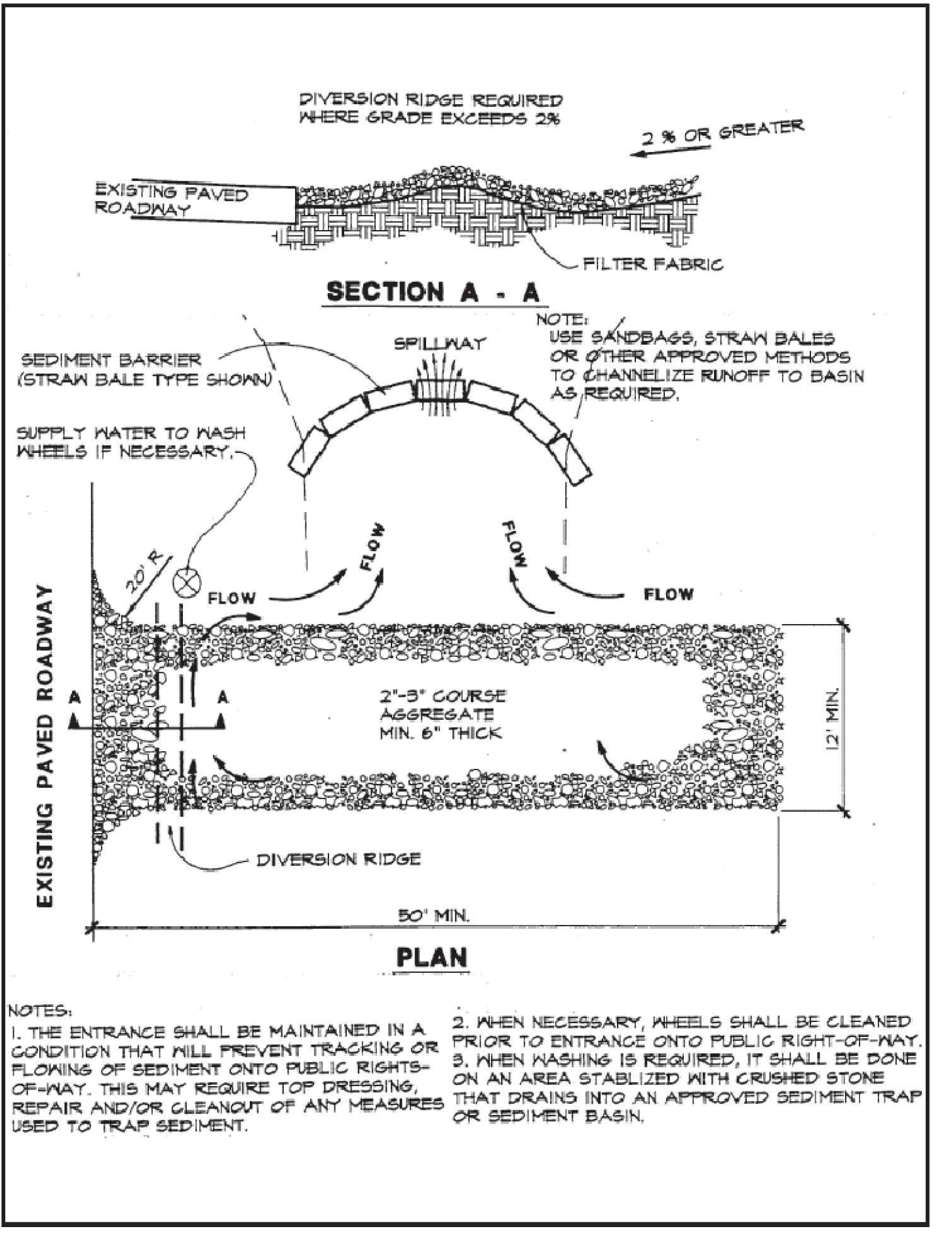
INSPECTION
 QUALIFIED PERSONNEL FROM THE CONTRACTOR SHALL INSPECT THE FOLLOWING ITEMS DAILY THROUGHOUT THE CONSTRUCTION PERIOD. PREPARATION OF THE CONTRACTOR'S WEEKLY REPORT OF INSPECTION, MAINTENANCE AND REPAIRS FOR THE CONTROL AND ABATEMENT OF EROSION AND WATER POLLUTION SHALL BE INCLUDED IN THE COST OF THE RESPECTIVE PAY ITEMS. WHERE SITES HAVE BEEN FINALLY STABILIZED, INSPECTIONS REPORTS SHALL BE COMPILED WEEKLY.



SILT FENCE
SCALE: N.T.S.
REFER TO FLORIDA EROSION
AND SEDIMENT CONTROL MANUAL



DROP INLET SEDIMENT BARRIER
SCALE: N.T.S.
REFER TO FLORIDA EROSION
AND SEDIMENT CONTROL MANUAL



TEMPORARY GRAVEL CONSTRUCTION ENTRANCE
SCALE: N.T.S.
REFER TO FLORIDA EROSION
AND SEDIMENT CONTROL MANUAL

ER	NO	DATE	REVISION	BY	CH	COR	APP

DATE: AUGUST 2026	SCALE: AS NOTED
POWER DELIVERY	DR: N/A CH: WDH APPROVED:
FPL	ER: WDH COR: N/A

MARTIN PLANT - 500KV LAYDOWN YARD
MARTIN COUNTY, FLORIDA
**STORMWATER POLLUTION
PREVENTION PLAN DETAILS**

PERMIT DRAWINGS
DATE: AUGUST 18, 2026

SOMI NO. 26008
FPL NO.
SHEET 11 OF 11
E-MAR02

Attachment C
Stormwater Report

Florida Power & Light
Martin Plant 500kV Laydown Yard
21900 SW Warfield Boulevard
Indiantown, Florida 34956
Martin County

Stormwater Management Report

Presented on behalf of:



Florida Power & Light, Co.
700 Universe Boulevard (TS4/JW)
Juno Beach, Florida 33408

Presented by:



SOMI Engineering, LLC
2332 Galiano Street 2nd Floor
Coral Gables, FL 33134
(561) 901-9255

Date: August 18, 2026
SOMI Project No. 26008

Stormwater Management Report

For:

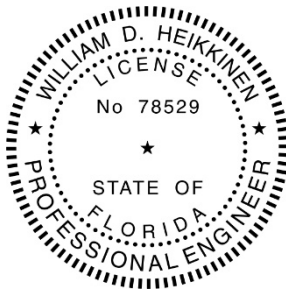
**Florida Power & Light
Martin Plant 500kV Laydown Yard**

21900 SW Warfield Boulevard
Indiantown, Florida 34956
Martin County

Presented on behalf of:



Florida Power & Light, Co.
700 Universe Boulevard (TS4/JW)
Juno Beach, Florida 33408



This item has been digitally signed and sealed by William D. Heikkinen, P.E. on August 18, 2026.

Printed Copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

William D. Heikkinen, P.E.
Florida Professional Engineer License No. 78529
SOMI Engineering, LLC
2332 Galiano Street 2nd Floor
Coral Gables, FL 33134
(561) 901-9255
Florida Certificate of Authorization No. 32647

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Project Description

The Florida Power & Light (FPL) Martin Plant 500kV Laydown Yard project is located at 21900 SW Warfield Boulevard, Indiantown, Florida, within Sections 20, 21, 28, and 29, Township 39 South, Range 38 East, at the existing FPL Martin Power Plant. A general Site Location Map is provided in **Figure 1**. According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 12085C0250G (effective March 16, 2015), the project site is located within Flood Zone X, as shown in **Figure 2**.

The proposed laydown yard is located within a portion of the previously permitted Martin Solar Energy Center, which lies within the Site Certification Area (SCA) of the Martin Power Plant. The former solar energy facility was decommissioned in 2024, and the associated solar infrastructure has been removed. The existing solar basins remain in place and are proposed to be repurposed to support future operational and construction activities within the power plant.

The proposed project consists of the redevelopment of Solar Basins S2, S3 and portions of Solar Basins S7 and S8 into stabilized laydown yard areas. The laydown yards will consist of compacted native materials to provide durable equipment and material storage areas. Existing drainage patterns will be maintained through the use of proposed interior stormwater conveyance ditches that route runoff to existing header ditches with control structures for each basin. Aggregate access drives will also be constructed to provide vehicular access to the proposed laydown yard areas.

This Stormwater Management Report has been prepared to evaluate the hydrologic and hydraulic performance of the proposed improvements within Solar Basin S2, S3, and portions of Solar Basins S7 and S8, demonstrate compliance with applicable South Florida Water Management District (SFWMD) stormwater management criteria, and present the supporting calculations and modeling analyses used in the design of the proposed stormwater management system. The scope of this report is limited to the proposed modifications within Solar Basins S2, S3, S7, and S8. The stormwater management evaluation is limited to the solar basins within the limits of the proposed work. The hydrologic and hydraulic performance at the control structure for each basin within the project scope was reviewed to confirm that post-development water quality treatment, stage-storage performance, and discharge rates are maintained at levels consistent with the existing conditions.

Stormwater Management Criteria

The project will be required to meet the Rules & Regulations of the Village of Indiantown (VOI), Martin County (MC), the South Florida Water Management District (SFWMD) and the Florida Department of Environmental Protection (FDEP). The following design criteria have been established for the project:

- Water Quality Treatment: Provide treatment for runoff equal to 1 inch over the total project area or 2.5 inches multiplied by the impervious area percentage, whichever is greater.
- Stormwater Retention – SFWMD/FDEP Requirement: Post-development stormwater runoff rates and volumes shall not exceed pre-development runoff rates and volumes for the 25-year, 72-hour design storm.

Existing Conditions

The existing Martin Solar Energy Center was completed in 2010. The plant was permitted under Site Certification No. PA 89-27. The site certification includes two nominal 500 MW 2-on-1 combined cycle units (Units 3 and 4), each having two combustion turbines with associated heat recovery steam generators and a single steam turbine-electric generator; a nominal 1,100 MW 4-on-1 combined cycle unit (Unit 8), comprised of four combustion turbines with associated heat recovery steam generators and a single steam turbine-electric generator; the 75 MW Martin Next Generation Solar Energy Center; and associated support facilities. In 2024, FPL initiated decommissioning of the Solar Energy Center with the existing solar basins remain in place and are proposed to be repurposed to support future operational and construction activities within the power plant.

In 2013, MDO Engineering, Inc. (MDO) performed an analysis of the existing stormwater management system to confirm the required water quality treatment prior to discharge and verify compliance with applicable South Florida Water Management District (SFWMD) criteria and SCA requirements. For purposes of this Stormwater Management Report, the conditions evaluated and documented in the 2013 MDO analysis were considered representative of the existing conditions. The results of the 2013 MDO Report for the four (4) solar basins are summarized below. Refer to **Figure 3 – 2013 MDO Basin Map**.

Solar Basin S2 – 58.1-acres (former solar yard)

- 34.4-acre impervious area, 23.7-acre pervious area
- Control elevation = 26.0' NAVD (CS-02 orifice invert)
- Twenty (20) interior drainage ditches routing stormwater north towards control structure CS-02.
- Existing Discharge Rate through CS-02 = 39.66 CFS (MDO Report)

Solar Basin S3 - 57.7-acres (former solar yard)

- 34.4-acre impervious area, 23.3-acre pervious area
- Control elevation = 27.0' NAVD (CS-03 orifice invert)
- Twenty (20) interior drainage ditches routing stormwater north towards control structure CS-03.
- Existing Discharge Rate through CS-03 = 28.24 CFS (MDO Report)

Solar Basin S7 - 61.0-acres (former solar yard)

- 30.9-acre impervious area, 30.1-acre pervious area
- Control elevation = 28.0' NAVD (CS-07 orifice invert)
- Twenty-one (21) interior drainage ditches routing stormwater south towards control structure CS-07.
- Existing Discharge Rate through CS-07 = 35.35 CFS (MDO Report)

Solar Basin S8 - 62.3-acres (former solar yard)

- 37.9-acre impervious area, 24.4-acre pervious area
- Control elevation = 28.5 NAVD (CS-08 orifice invert)

- Twenty (20) interior drainage ditches routing stormwater south towards control structure CS-08.
- Existing Discharge Rate through CS-08 = 18.76 CFS (MDO Report)
- Existing Discharge Rate through Dual 18" Pipes to Basin 7 = 1.31 CFS (MDO Report)

Existing Condition information was obtained from the Martin Plant (PMR) Storm Water Analysis Report, prepared by MDO Engineering, Inc. (Montes De Oca, PE) and Patrick J. LaConte, PE, dated December 2, 2013.

Post-Development Conditions

For the post development condition, the 239.1-acre project area was evaluated as four (4) drainage basins. Refer to **Figure 4 - Post-Development Site Plan**. The post-development stormwater management design consists of the following:

Basin S2 – 58.1-acres

- 6.74-acre impervious area, 51.36-acre pervious area (CN = 88)
- Six (6) dry retention ditches routing stormwater north towards control structure CS-02.
- Control elevation = 26.0' NAVD (CS-02 orifice invert)
- Increase CS-02 Rim Elevation 6" to 29.3' NAVD.
- Total Basin S2 Storage = 53.52 acre-ft

Basin S3 - 57.7-acres

- 16.06-acre impervious area, 41.64-acre pervious area (CN = 88)
- Six (6) dry retention ditches routing stormwater north towards control structure CS-03.
- Control elevation = 27.0' NAVD (CS-03 orifice invert)
- Increase CS-03 Rim Elevation 6" to 30.2' NAVD.
- Total Basin S3 Storage = 53.56 acre-ft

Basin S7 - 61.0-acres

- 4.66-acre impervious area, 56.34-acre pervious area (CN = 85)
- Four (4) dry retention ditches routing stormwater south towards control structure CS-07.
- Control elevation = 28.0' NAVD (CS-07 orifice invert)
- Increase CS-07 Rim Elevation 6" to 31.2' NAVD.
- The southern portion of Solar Basin S7 will remain undisturbed, except for a proposed spreader swale along the south side of the asphalt-paved access drive. The swale is intended to provide connectivity between the proposed drainage basins and the existing stormwater conveyance ditches to the south.
- Solar infrastructure from the southern portion of Solar Basin S7 was removed from the site in 2024 with native material and existing conveyance ditches remaining and drainage flowing to the south consistent with the existing condition. This area with the exception of the asphalt paved access was considered pervious in calculations and modeling.
- Total Basin S7 Storage = 47.79 acre-ft

Basin S8 - 62.3-acres

- 5.27-acre impervious area, 57.03-acre pervious area (CN = 84)
- Six (6) dry retention ditches routing stormwater south towards control structure CS-08.
- Control elevation = 28.5 NAVD (CS-08 orifice invert)
- Increase CS-08 Rim Elevation 12" to 32.0' NAVD.
- The southern portion of Solar Basin S8 will remain undisturbed, except for a proposed spreader swale along the south side of the asphalt-paved access drive. The swale is intended to provide connectivity between the proposed drainage basins and the existing stormwater conveyance ditches.
- Solar infrastructure from the southern portion of Solar Basin S8 was removed from the site in 2024 with native material and existing conveyance ditches remaining and drainage flowing to the south consistent with the existing condition. This area with the exception of the asphalt paved access was considered pervious in calculations and modeling.
- Total Basin S8 Storage = 51.69 acre-ft

Stormwater Analysis

The proposed stormwater management system was modeled using Advanced Interconnected Channel and Pond Routing (ICPR/StormWise). The Natural Resources Conservation Service (NRCS) Unit Hydrograph Method, commonly known as the SCS TR-20 Method, was selected for runoff estimation. In accordance with South Florida Water Management District (SFWMD) guidelines, a unit hydrograph peak factor of 256 was used with the SCS TR-20 method, reflecting the flat terrain and slow-draining soil characteristics typical of South Florida watersheds. The following sections provide a summary of the data utilized in the design, along with the methods used for data collection and analysis.

Control Elevation

Control elevations for the site's stormwater management system were established based on existing, as-built conditions rather than a single sitewide design criterion. Each basin's outfall discharge structure was incorporated into the hydrologic/hydraulic model using its orifice invert elevation. Because each basin discharges through an independently constructed control structure, control elevations vary across the site, reflecting differences in grading and outfall configuration rather than a departure from a uniform design standard.

Rainfall Amounts

Rainfall depths used for the hydrologic analysis were obtained from the NOAA Atlas 14 Point Precipitation Frequency Estimate Tables. These updated precipitation depths are greater than those utilized in the 2013 MDO Report and represent the current design rainfall criteria. NOAA Atlas 14 Point Precipitation Tables are provided as **Appendix B**:

Table 1 – Rainfall Data

Rainfall Event	Rainfall Depth (in)
25-year, 24-hour	7.96
25-year, 72-hour	9.09
100-year, 72-hour	12.00

Curve Numbers (CN) and Time of Concentration (T_c)

Based on the USDA-NRCS Web Soil Survey, the predominant soil type within the project area consists of Waveland and Immokalee fine sands, which account for approximately 43.9% of the mapped Area of Interest (**Appendix C**). For purposes of the post-development hydrologic analysis, Hydrologic Soil Group D (HSG D) was utilized for Curve Number (CN) determination.

Post-development CNs were developed in accordance with NRCS TR-55, Table 2-2a, Runoff Curve Numbers for Urban Areas. Impervious areas were assigned a CN of 98. Compacted native material was considered representative of open space in poor hydrologic condition and assigned a CN of 89, reflecting the reduced infiltration and increased runoff potential associated with compaction. Remaining pervious areas were considered open space in good hydrologic condition and assigned a CN of 80.

A composite CN was calculated for each post-development drainage basin by area-weighting the applicable CN values based on the respective areas of impervious surface, compacted native material, and pervious surface within each basin.

Time of Concentration (T_c) values for the existing and post-development conditions were developed based on the hydraulic characteristics of the contributing drainage areas and associated flow paths. T_c values and assumptions were maintained consistently with the methodology utilized in the previous Martin Power Plant MDO Report, where applicable. The resulting T_c values for the existing and post-development conditions are provided in **Appendix A**.

Summary of Results

Water Quality

VOI, MC, SFWMD, and FDEP require that all projects meet water quality standards. For wet detention systems, 1-inch of runoff from the entire site area or 2.5-inches times the percent impervious, whichever is greater, must be detained on site. Dry detention systems must provide 75 percent of the required wet detention volume and must maintain the control elevation at least one foot above the seasonal high groundwater elevation. Dry retention systems must provide 50 percent of the required wet detention volume.

For the post-development condition, the controlling minimum water quality treatment volume is as follows in **Table 2**:

Table 2 – Water Quality

Drainage Basin	Required WQ Volume (acre-ft)	Method of Treatment	WQ Volume Provided (acre-ft)
S2	4.84	Dry Retention Swales	53.52
S3	4.81	Dry Retention Swales	53.56
S7	5.08	Dry Retention Swales	47.79
S8	5.19	Dry Retention Swales	51.69

Water Quality calculations are provided in **Appendix A**.

Water Quantity

Per VOI, MC, SFWMD, and FDEP, the post-development runoff shall not exceed the pre-development runoff during the 25-year, 72-hour storm event.

The existing condition and post-development runoff rates for the 25-year, 72-hour storm are provided in **Table 3**:

Table 3 – Existing vs Post Summary

Basin/Rainfall Event	Existing	Post-Development
Project Area (acres)	239.10	239.10
S2 Runoff (cfs) (25yr-72hr)	39.66	35.97
S3 Runoff (cfs) (25yr-72hr)	28.24	24.31
S7 Runoff (cfs) (25yr-72hr)	35.35	22.55
S8 Runoff (cfs) (25yr-72hr)	20.07 ¹	10.72 ¹

Note 1: Considers discharge through both CS-08 and dual 18" pipes to Basin 7

As demonstrated in **Table 3**, the post-development runoff rates for the 25-year, 72-hour storm event do not exceed the corresponding existing runoff rates. Therefore, the proposed improvements are not anticipated to adversely impact downstream basins.

A model using ICPR was run for stormwater routing and to determine the peak stage(s) for the site. Storage in the structures and pipes were not accounted for in the model. The data input/output for the post-development stormwater models is provided in **Appendix D**.

Table 4 – Post-Development Peak Stages

Basin/Rainfall Event	Post-Development Peak Stage (ft-NAVD)	Proposed Elevation (ft-NAVD)
S2 (25yr-72hr)	30.10	31.00 (Top of Laydown)
S3 (25yr-72hr)	30.86	32.00 (Top of Laydown)
S7 (25yr-72hr)	31.78	32.50 (Top of Laydown)
S8 (25yr-72hr)	32.30	33.00 (Top of Laydown)

Per the ICPR model results in **Table 4**, the peak stages for the 25-year, 72-hour storm event remain below the top elevation of the proposed laydown areas.

Conclusion

Based on the analyses and information presented in this Stormwater Management Report, the proposed stormwater management system has been designed to provide the required water quality treatment and stormwater attenuation for the proposed improvements. The results demonstrate that the proposed design reduces post-development discharge rates to the downstream basins when compared to the existing condition, while providing adequate storage and attenuation of stormwater runoff. Accordingly, the proposed improvements are not anticipated to adversely impact the downstream stormwater management system.

References

Boyle Engineering. (2009). *Florida Power & Light – The Martin Solar Energy Center Construction Plans & Specifications*. Prepared for Florida Power & Light Company.

MDO Engineering, Inc. (2013). *Martin Plant (PMR) Storm Water Analysis Report*. Prepared by Montes De Oca, P.E. and Patrick J. LaConte, P.E., December 2, 2013.

FIGURES

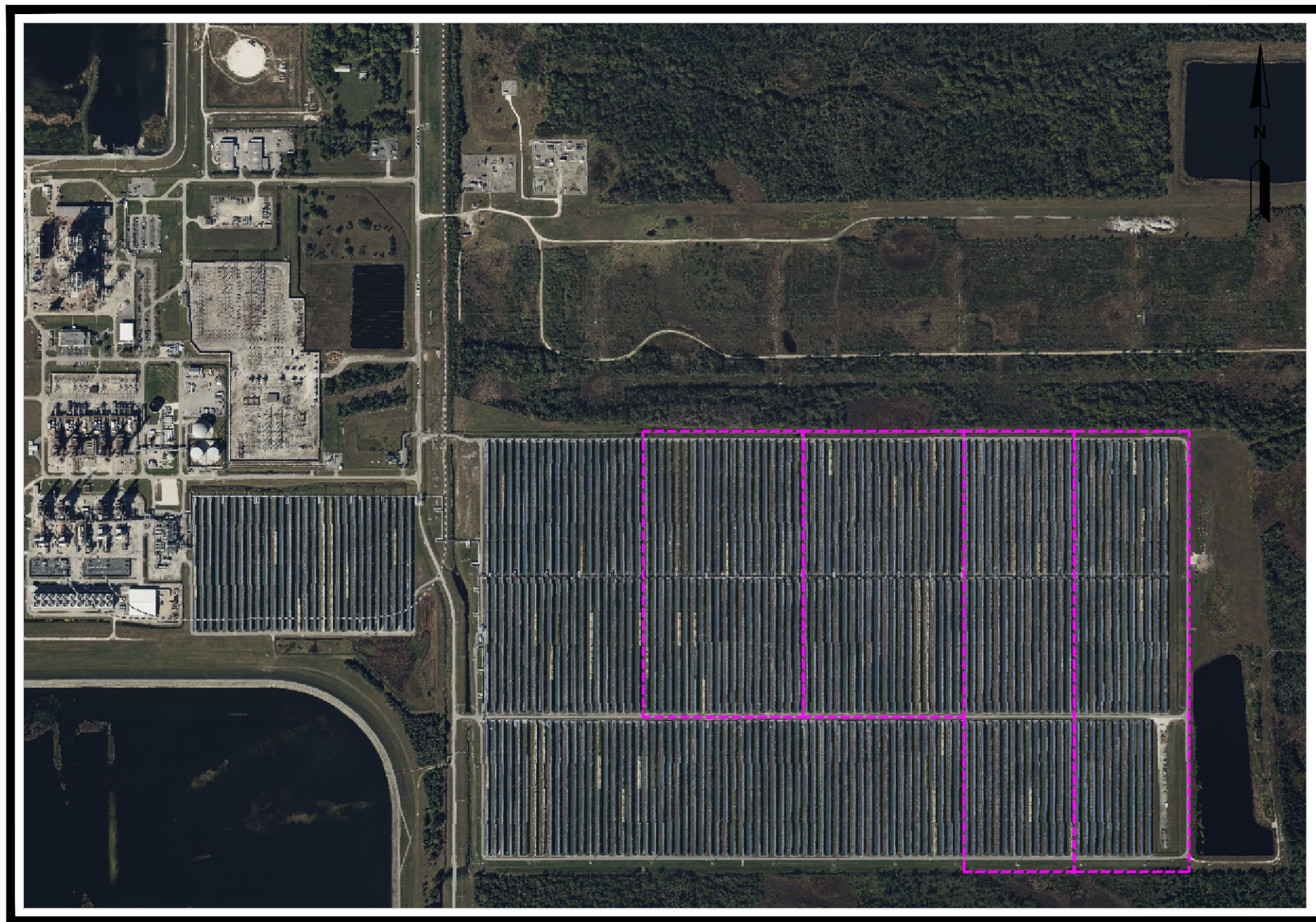
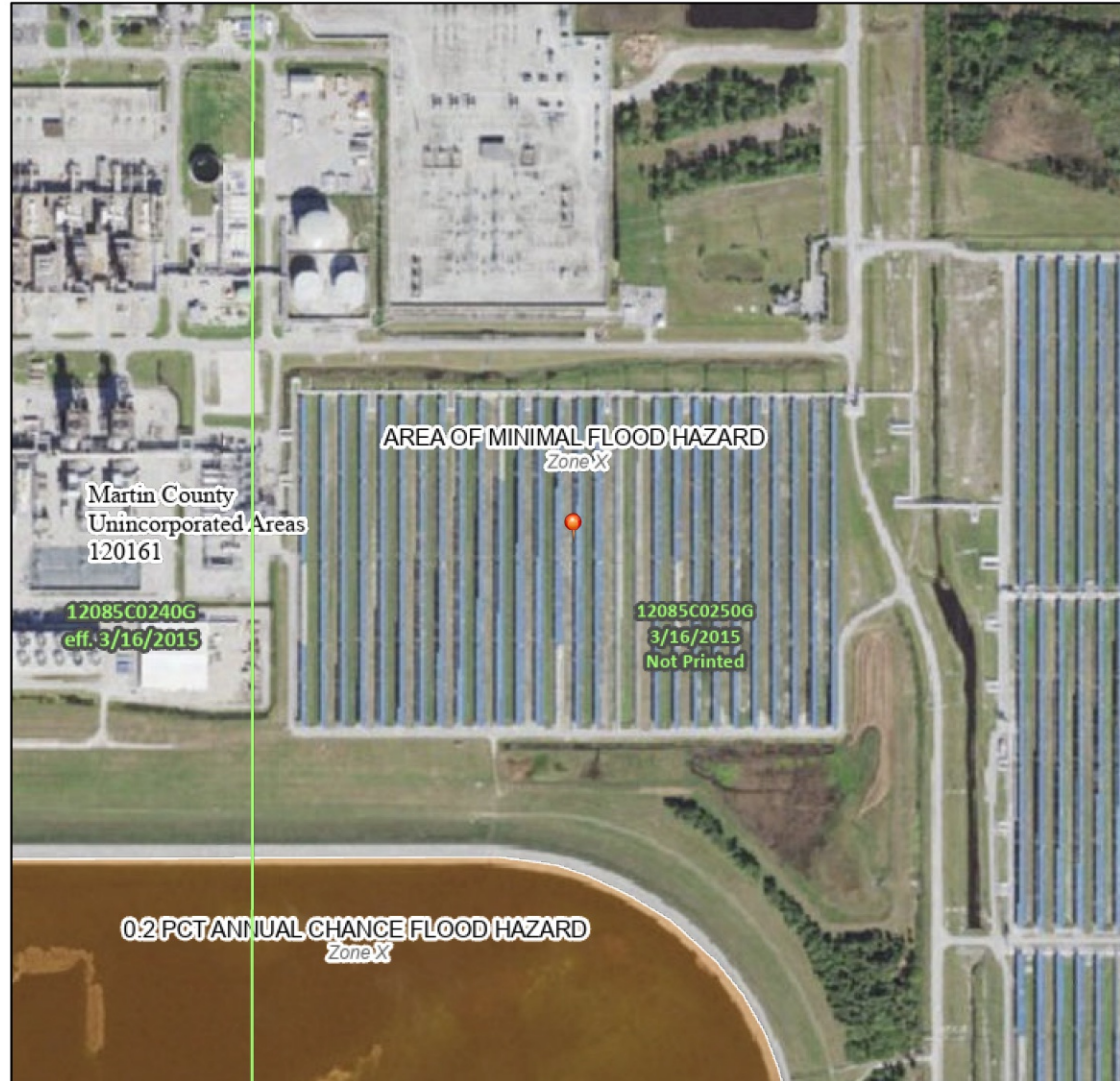


FIGURE 1 - SITE LOCATION MAP
FPL MARTIN PLANT 500kV LAYDOWN YARD

National Flood Hazard Layer FIRMette



80°33'53"W 27°3'27"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

80°33'15"W 27°2'55"N

Basemap Imagery Source: USGS National Map 2023

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 7/20/2026 at 7:57 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

FIGURE 2 - FEMA FIRM
FPL MARTIN PLANT 500kV LAYDOWN YARD

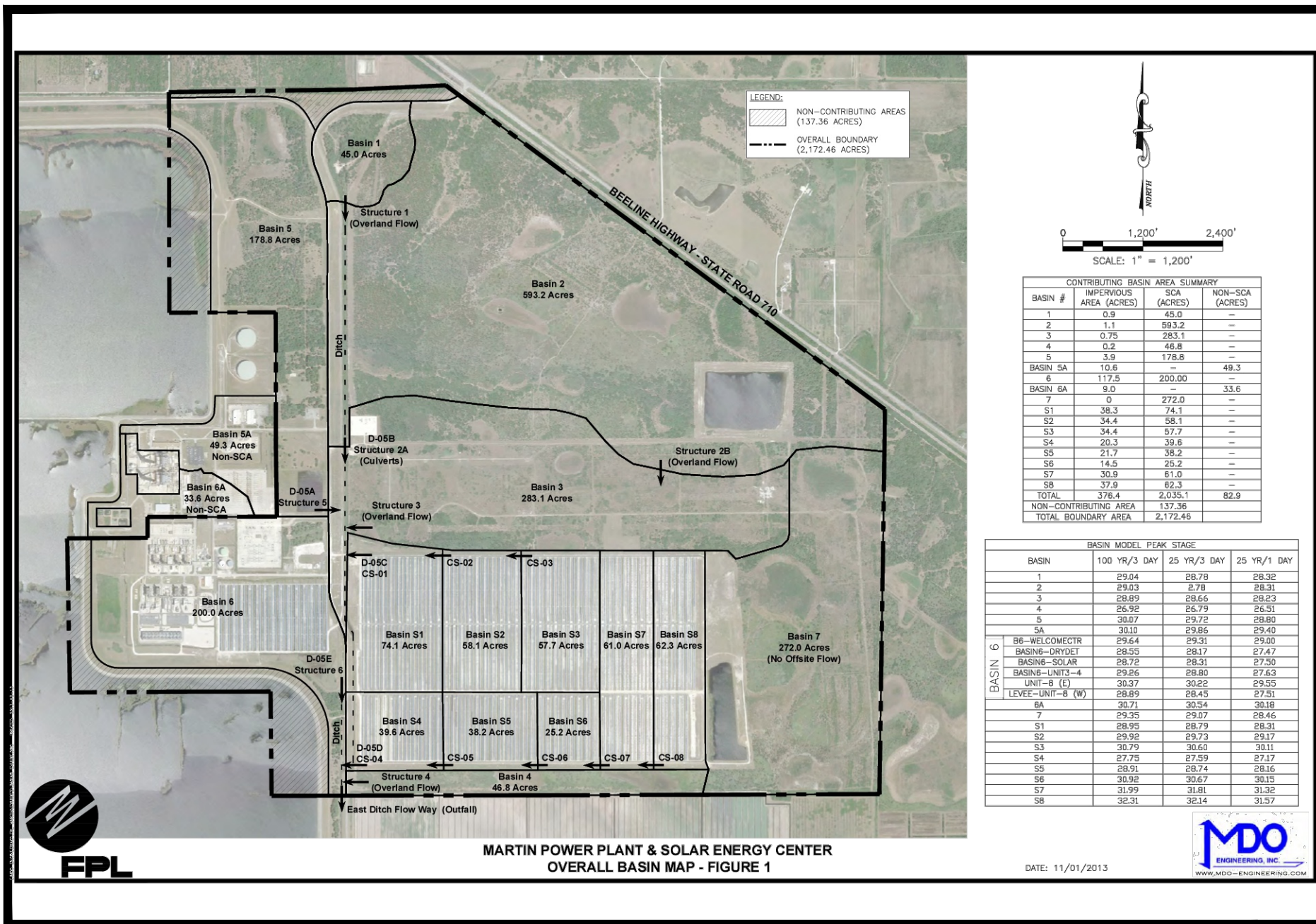
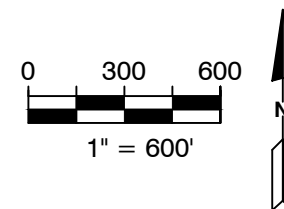
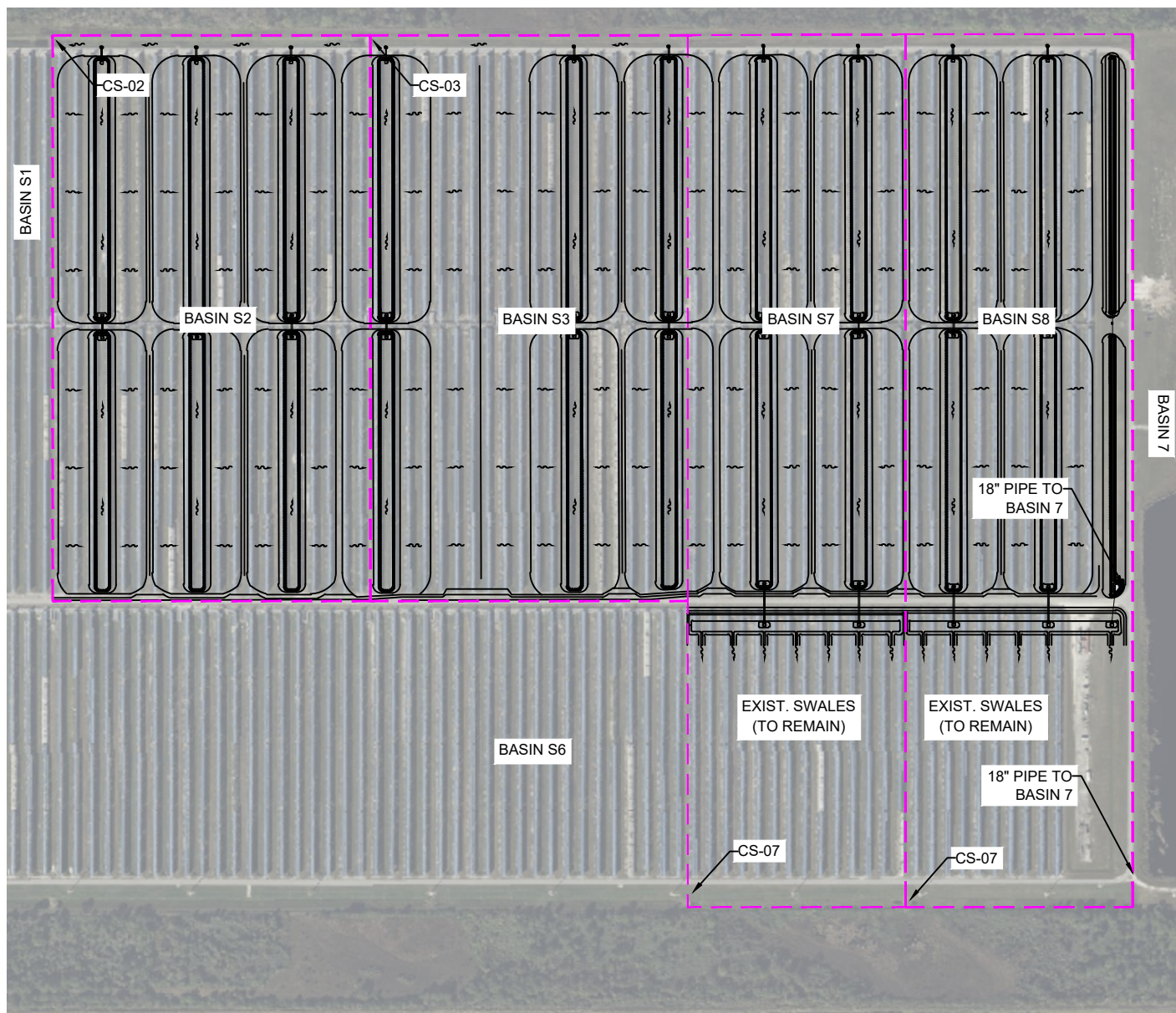


FIGURE 3 - 2013 MDO OVERALL BASIN MAP
FPL MARTIN 500kV LAYDOWN YARD



LEGEND:

--- BASIN BOUNDARY

POST-DEVELOPMENT		
BASIN	AREA (AC)	IMPERVIOUS (AC)
BASIN S2	58.10	6.74
BASIN S3	57.70	16.06
BASIN S7	61.00	4.66
BASIN S8	62.30	5.27

NOTES:

1. REFER TO APPENDIX A - STORMWATER MANAGEMENT CALCULATIONS

FIGURE 4 - POST-DEVELOPMENT SITE PLAN
FPL MARTIN 500kV LAYDOWN YARD

APPENDIX A

STORMWATER MANAGEMENT CALCULATIONS

STORMWATER CALCULATIONS - POST DEVELOPMENT BASIN S2**PROJECT:**

FPL Martin Plant 500kV Laydown Yard
 Florida Power & Light Martin Solar Energy Center

STORMWATER MANAGEMENT DATA:**Post-Development Basin Area Breakdown:**

Basin Area	Acres
Project Area - Basin S2	58.10
Impervious Area	6.74
Pervious Area	51.36

Rainfall Data:

25-year, 24-hour Storm Event (Total Rainfall)	7.96	inches
25-year, 72-hour Storm Event (Total Rainfall)	9.09	inches
100-year, 72-hour Storm Event (Total Rainfall)	12.00	inches

STORMWATER BASIN DATA:**Site Data:**

Impervious Area:	6.74 acres
Pervious Area:	51.36 acres
Project Area:	58.10 acres
% Impervious:	12 %
% Pervious:	88 %
Control Elevation	26.00 ft NAVD
Avg. Depth to Seasonal High Water Table:	1.0 ft

Curve Number (Composite)**Hydrologic Soil Group D (Waveland and Immokalee fine sands)
TR-55 Table 2-2**

Description	Area		CN
Impervious (Access)	6.74	acres	98
Compacted Native Material	35.51	acres	89
Pervious (Dry Ret./Sod/Seed)	15.85	acres	80
Total =	58.10	acres	

Composite Curve Number, $CN = \text{Sum}(Cn_i \times A_i) / A_t$

Composite Curve Number, CN = 88

Soil Storage

$$S = (1000/CN) - 10 \quad S = 1.42$$

1. Provide Water Quality for:

- | | | |
|---|---------------------|-----------------|
| a) 1-inch over the entire project area | 4.84 acre-ft | *Governs |
| b) 2.5-inch times the percentage impervious | 1.40 acre-ft | |

2. Retention of the 25-year, 24-hour storm event

Runoff (Q) from the 25-year, 24-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 7.96 \text{ inches} \\ S = & 1.42 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 24-hour} \end{array}$$

$$Q = 6.48 \text{ inches} \quad \underline{\underline{Q = 31.38 \text{ acre-ft}}}$$

3. Retention of the 25-year, 72-hour storm event

Runoff (Q) from the 25-year, 72-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 9.09 \text{ inches} \\ S = & 1.42 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 72-hour} \end{array}$$

$$Q = 7.59 \text{ inches} \quad \underline{\underline{Q = 36.73 \text{ acre-ft}}}$$

4. Retention of the 100-year, 72-hour storm event

Runoff (Q) from the 100-year, 24-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 12.00 \text{ inches} \\ S = & 1.42 \text{ inches} \end{array} \quad \begin{array}{l} 100\text{-year, 72-hour} \end{array}$$

$$Q = 10.45 \text{ inches} \quad \underline{\underline{Q = 50.61 \text{ acre-ft}}}$$

5. Time of Concentration**20 minutes****6. Post-Development Storage Available****Laydown Swale 1 (NW)**

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.98	0.00	0.00
28.0	1.16	1.07	1.07
29.0	1.35	1.26	2.33
30.0	2.26	1.81	4.13
31.0	7.41	4.83	8.96

Laydown Swale 2 (N Middle)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.98	0.00	0.00
28.0	1.16	1.07	1.07
29.0	1.35	1.26	2.33
30.0	2.26	1.81	4.13
31.0	7.41	4.83	8.96

Laydown Swale 3 (NE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.98	0.00	0.00
28.0	1.16	1.07	1.07
29.0	1.35	1.26	2.33
30.0	2.26	1.81	4.13
31.0	7.41	4.83	8.96

Laydown Swale 4 (SW)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.96	0.00	0.00
28.0	1.15	1.06	1.06
29.0	1.33	1.24	2.30
30.0	2.23	1.78	4.08
31.0	7.37	4.80	8.88

Laydown Swale 5 (S Middle)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.96	0.00	0.00
28.0	1.15	1.06	1.06
29.0	1.33	1.24	2.30
30.0	2.23	1.78	4.08
31.0	7.37	4.80	8.88

Laydown Swale 6 (SE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	0.96	0.00	0.00
28.0	1.15	1.06	1.06
29.0	1.33	1.24	2.30
30.0	2.23	1.78	4.08
31.0	7.37	4.80	8.88

Total Laydown Basin S2 Storage

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
27.0	5.82	0.00	0.00
28.0	6.93	6.38	6.38
29.0	8.04	7.49	13.86
30.0	13.47	10.76	24.62
31.0	44.34	28.90	53.52

STORMWATER CALCULATIONS - POST DEVELOPMENT BASIN S3**PROJECT:**

FPL Martin Plant 500kV Laydown Yard
 Florida Power & Light Martin Solar Energy Center

STORMWATER MANAGEMENT DATA:**Post-Development Basin Area Breakdown:**

Basin Area	Acres
Project Area - Basin S3	57.70
Impervious Area	16.06
Pervious Area	41.64

Rainfall Data:

25-year, 24-hour Storm Event (Total Rainfall)	7.96	inches
25-year, 72-hour Storm Event (Total Rainfall)	9.09	inches
100-year, 72-hour Storm Event (Total Rainfall)	12.00	inches

STORMWATER BASIN DATA:**Site Data:**

Impervious Area:	16.06 acres
Pervious Area:	41.64 acres
Project Area:	57.70 acres
% Impervious:	28 %
% Pervious:	72 %
Control Elevation	27.00 ft NAVD
Avg. Depth to Seasonal High Water Table:	1.0 ft

Curve Number (Composite)**Hydrologic Soil Group D (Waveland and Immokalee fine sands)
TR-55 Table 2-2**

Description	Area		CN
Impervious (Access)	16.06	acres	98
Compacted Native Material	21.46	acres	89
Pervious (Dry Ret./Sod/Seed)	20.18	acres	80
Total =	57.70	acres	

Composite Curve Number, $CN = \text{Sum}(Cn_i \times A_i) / A_t$

Composite Curve Number, CN = 88

Soil Storage

$$S = (1000/CN) - 10 \quad S = 1.32$$

1. Provide Water Quality for:

- | | | |
|---|---------------------|-----------------|
| a) 1-inch over the entire project area | 4.81 acre-ft | *Governs |
| b) 2.5-inch times the percentage impervious | 3.35 acre-ft | |

2. Retention of the 25-year, 24-hour storm event

Runoff (Q) from the 25-year, 24-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 7.96 \text{ inches} \\ S = & 1.32 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 24-hour} \end{array}$$

$$Q = 6.57 \text{ inches} \quad \underline{\underline{Q = 31.60 \text{ acre-ft}}}$$

3. Retention of the 25-year, 72-hour storm event

Runoff (Q) from the 25-year, 72-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 9.09 \text{ inches} \\ S = & 1.32 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 72-hour} \end{array}$$

$$Q = 7.68 \text{ inches} \quad \underline{\underline{Q = 36.93 \text{ acre-ft}}}$$

4. Retention of the 100-year, 72-hour storm event

Runoff (Q) from the 100-year, 72-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 12.00 \text{ inches} \\ S = & 1.32 \text{ inches} \end{array} \quad \begin{array}{l} 100\text{-year, 72-hour} \end{array}$$

$$Q = 10.55 \text{ inches} \quad \underline{\underline{Q = 50.74 \text{ acre-ft}}}$$

5. Time of Concentration**20 minutes****6. Post-Development Storage Available****Laydown Swale 1 (NW)**

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.98	0.00	0.00
29.0	1.16	1.07	1.07
30.0	1.35	1.26	2.33
31.0	2.26	1.81	4.13
32.0	7.41	4.83	8.96

Laydown Swale 2 (N Middle)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.98	0.00	0.00
29.0	1.16	1.07	1.07
30.0	1.35	1.26	2.33
31.0	2.26	1.81	4.13
32.0	7.41	4.83	8.96

Laydown Swale 3 (NE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.98	0.00	0.00
29.0	1.16	1.07	1.07
30.0	1.35	1.26	2.33
31.0	2.26	1.81	4.13
32.0	7.41	4.83	8.96

Laydown Swale 4 (SW)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.97	0.00	0.00
29.0	1.15	1.06	1.06
30.0	1.33	1.24	2.30
31.0	2.23	1.78	4.08
32.0	7.39	4.81	8.89

Laydown Swale 5 (S Middle)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.97	0.00	0.00
29.0	1.15	1.06	1.06
30.0	1.33	1.24	2.30
31.0	2.23	1.78	4.08
32.0	7.39	4.81	8.89

Laydown Swale 6 (SE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	0.97	0.00	0.00
29.0	1.15	1.06	1.06
30.0	1.33	1.24	2.30
31.0	2.23	1.78	4.08
32.0	7.39	4.81	8.89

Total Laydown Basin S3 Storage

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
28.0	5.84	0.00	0.00
29.0	6.94	6.39	6.39
30.0	8.04	7.49	13.88
31.0	13.47	10.76	24.63
32.0	44.40	28.93	53.56

STORMWATER CALCULATIONS - POST DEVELOPMENT BASIN S7**PROJECT:**

FPL Martin Plant 500kV Laydown Yard
 Florida Power & Light Martin Solar Energy Center

STORMWATER MANAGEMENT DATA:**Post-Development Basin Area Breakdown:**

Basin Area	Acres
Project Area - Basin S7	61.00
Impervious Area	4.66
Pervious Area	56.34

Rainfall Data:

25-year, 24-hour Storm Event (Total Rainfall)	7.96	inches
25-year, 72-hour Storm Event (Total Rainfall)	9.09	inches
100-year, 72-hour Storm Event (Total Rainfall)	12.00	inches

STORMWATER BASIN DATA:**Site Data:**

Impervious Area:	4.66 acres
Pervious Area:	56.34 acres
Project Area:	61.00 acres
% Impervious:	8 %
% Pervious:	92 %
Control Elevation	28.00 ft NAVD
Avg. Depth to Seasonal High Water Table:	1.0 ft

Curve Number (Composite)**Hydrologic Soil Group D (Waveland and Immokalee fine sands)
TR-55 Table 2-2**

Description	Area		CN
Impervious (Access)	4.66	acres	98
Compacted Native Material	24.85	acres	89
Pervious (Dry Ret./Sod/Seed)	31.49	acres	80
Total =	61.00	acres	

Composite Curve Number, $CN = \text{Sum}(Cn_i \times A_i) / A_t$

Composite Curve Number, CN = 85

Soil Storage

$$S = (1000/CN) - 10 \quad S = 1.76$$

1. Provide Water Quality for:

- | | | |
|---|---------------------|-----------------|
| a) 1-inch over the entire project area | 5.08 acre-ft | *Governs |
| b) 2.5-inch times the percentage impervious | 0.97 acre-ft | |

2. Retention of the 25-year, 24-hour storm event

Runoff (Q) from the 25-year, 24-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 7.96 \text{ inches} \\ S = & 1.76 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 24-hour} \end{array}$$

$$Q = 6.18 \text{ inches} \quad \underline{\underline{Q = 31.41 \text{ acre-ft}}}$$

3. Retention of the 25-year, 72-hour storm event

Runoff (Q) from the 25-year, 72-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 9.09 \text{ inches} \\ S = & 1.76 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 72-hour} \end{array}$$

$$Q = 7.27 \text{ inches} \quad \underline{\underline{Q = 36.98 \text{ acre-ft}}}$$

4. Retention of the 100-year, 72-hour storm event

Runoff (Q) from the 100-year, 24-hour storm event:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad \begin{array}{ll} P = & 12.00 \text{ inches} \\ S = & 1.76 \text{ inches} \end{array} \quad \begin{array}{l} 100\text{-year, 72-hour} \end{array}$$

$$Q = 10.12 \text{ inches} \quad \underline{\underline{Q = 51.44 \text{ acre-ft}}}$$

5. Time of Concentration**20 minutes****6. Post-Development Storage Available****Laydown Swale 1 (NW)**

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	1.07	0.00	0.00
29.5	1.16	0.56	0.56
30.5	1.35	1.26	1.81
31.5	2.26	1.81	3.62
32.5	7.41	4.83	8.45

Laydown Swale 2 (NE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	1.07	0.00	0.00
29.5	1.16	0.56	0.56
30.5	1.35	1.26	1.81
31.5	2.26	1.81	3.62
32.5	7.41	4.83	8.45

Laydown Swale 3 (SW)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	1.05	0.00	0.00
29.5	1.14	0.55	0.55
30.5	1.32	1.23	1.78
31.5	2.21	1.77	3.54
32.5	7.32	4.77	8.31

Laydown Swale 4 (SE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	1.05	0.00	0.00
29.5	1.14	0.55	0.55
30.5	1.32	1.23	1.78
31.5	2.21	1.77	3.54
32.5	7.32	4.77	8.31

Existing South Solar Swales to Remain (7 swales)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	0.91	0.00	0.00
29.5	1.38	0.57	0.57
30.5	2.33	1.86	2.43
31.5	3.29	2.81	5.24
32.5	14.78	9.04	14.27

Total Laydown Basin S7 Storage

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.0	5.15	0.00	0.00
29.5	5.98	2.78	2.78
30.5	7.67	6.83	9.61
31.5	12.23	9.95	19.56
32.5	44.24	28.23	47.79

STORMWATER CALCULATIONS - POST DEVELOPMENT BASIN S8**PROJECT:**

FPL Martin Plant 500kV Laydown Yard
 Florida Power & Light Martin Solar Energy Center

STORMWATER MANAGEMENT DATA:**Post-Development Basin Area Breakdown:**

Basin Area	Acres
Project Area - Basin S8	62.30
Impervious Area	5.27
Pervious Area	57.03

Rainfall Data:

25-year, 24-hour Storm Event (Total Rainfall)	7.96	inches
25-year, 72-hour Storm Event (Total Rainfall)	9.09	inches
100-year, 72-hour Storm Event (Total Rainfall)	12.00	inches

STORMWATER BASIN DATA:**Site Data:**

Impervious Area: 5.27 acres
 Pervious Area: 57.03 acres
 Project Area: 62.30 acres

% Impervious: 8 %
 % Pervious: 92 %

Control Elevation 28.50 ft NAVD

Avg. Depth to Seasonal High Water Table: 1.0 ft

Curve Number (Composite)**Hydrologic Soil Group D (Waveland and Immokalee fine sands)****TR-55 Table 2-2**

Description	Area	CN
Impervious (Access)	5.27 acres	98
Compacted Native Material	20.42 acres	89
Pervious (Dry Ret./Sod/Seed)	36.61 acres	80
Total =	62.30 acres	

Composite Curve Number, $CN = \text{Sum}(Cn_i \times A_i) / A_t$

Composite Curve Number, CN = 84

Soil Storage

$$S = (1000/CN) - 10 \quad S = 1.84$$

1. Provide Water Quality for:

- | | | |
|---|---------------------|-----------------|
| a) 1-inch over the entire project area | 5.19 acre-ft | *Governs |
| b) 2.5-inch times the percentage impervious | 1.10 acre-ft | |

2. Retention of the 25-year, 24-hour storm event

Runoff (Q) from the 25-year, 24-hour storm event:

$$Q = \frac{(P-0.2S)^2}{P+0.8S} \quad \begin{array}{ll} P = & 7.96 \text{ inches} \\ S = & 1.84 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 24-hour} \end{array}$$

$$Q = 6.11 \text{ inches} \quad \underline{\underline{Q = 31.73 \text{ acre-ft}}}$$

3. Retention of the 25-year, 72-hour storm event

Runoff (Q) from the 25-year, 72-hour storm event:

$$Q = \frac{(P-0.2S)^2}{P+0.8S} \quad \begin{array}{ll} P = & 9.09 \text{ inches} \\ S = & 1.84 \text{ inches} \end{array} \quad \begin{array}{l} 25\text{-year, 72-hour} \end{array}$$

$$Q = 7.20 \text{ inches} \quad \underline{\underline{Q = 37.40 \text{ acre-ft}}}$$

4. Retention of the 100-year, 72-hour storm event

Runoff (Q) from the 100-year, 24-hour storm event:

$$Q = \frac{(P-0.2S)^2}{P+0.8S} \quad \begin{array}{ll} P = & 12.00 \text{ inches} \\ S = & 1.84 \text{ inches} \end{array} \quad \begin{array}{l} 100\text{-year, 72-hour} \end{array}$$

$$Q = 10.05 \text{ inches} \quad \underline{\underline{Q = 52.15 \text{ acre-ft}}}$$

5. Time of Concentration**20 minutes****6. Post-Development Storage Available****Laydown Swale 1 (NW)**

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	1.07	0.00	0.00
30.0	1.16	0.56	0.56
31.0	1.37	1.27	1.82
32.0	2.29	1.83	3.65
33.0	7.41	4.85	8.50

Laydown Swale 2 (NE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	1.07	0.00	0.00
30.0	1.16	0.56	0.56
31.0	1.37	1.27	1.82
32.0	2.29	1.83	3.65
33.0	7.41	4.85	8.50

Laydown Swale 3 (SW)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	1.06	0.00	0.00
30.0	1.15	0.55	0.55
31.0	1.34	1.25	1.80
32.0	2.24	1.79	3.59
33.0	7.28	4.76	8.35

Laydown Swale 4 (SE)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	1.06	0.00	0.00
30.0	1.15	0.55	0.55
31.0	1.34	1.25	1.80
32.0	2.24	1.79	3.59
33.0	7.28	4.76	8.35

East Swale (N)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	0.18	0.00	0.00
30.0	0.36	0.14	0.14
31.0	0.53	0.45	0.58
32.0	1.11	0.82	1.40
33.0	1.84	1.48	2.88

East Swale (S)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	0.18	0.00	0.00
30.0	0.36	0.14	0.14
31.0	0.53	0.45	0.58
32.0	1.11	0.82	1.40
33.0	1.84	1.48	2.88

Existing South Solar Swales to Remain (6 swales)

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	0.78	0.00	0.00
30.0	1.19	0.49	0.49
31.0	2.00	1.60	2.09
32.0	2.82	2.41	4.50
33.0	12.67	7.75	12.24

Total Laydown Basin S8 Storage

Stage	Contour	Cumulative Storage	
(ft NAVD)	Area (ac)	Vol. (ac-ft)	Cum Vol. (acre-ft)
29.5	5.40	0.00	0.00
30.0	6.53	2.98	2.98
31.0	8.48	7.51	10.49
32.0	14.10	11.29	21.78
33.0	45.73	29.92	51.69

APPENDIX B

NOAA ATLAS 14 PRECIPITATION TABLES



NOAA Atlas 14, Volume 9, Version 2
Location name: Indiantown, Florida, USA*
Latitude: 27.0529°, Longitude: -80.5434°
Elevation: 32 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffrey Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.495 (0.406-0.617)	0.567 (0.464-0.707)	0.686 (0.559-0.857)	0.786 (0.637-0.987)	0.927 (0.726-1.20)	1.04 (0.794-1.37)	1.15 (0.850-1.55)	1.27 (0.896-1.76)	1.42 (0.968-2.03)	1.55 (1.02-2.24)
10-min	0.725 (0.594-0.904)	0.830 (0.679-1.04)	1.00 (0.818-1.26)	1.15 (0.933-1.45)	1.36 (1.06-1.76)	1.52 (1.16-2.00)	1.68 (1.24-2.27)	1.86 (1.31-2.57)	2.09 (1.42-2.97)	2.26 (1.50-3.28)
15-min	0.885 (0.725-1.10)	1.01 (0.828-1.26)	1.22 (0.998-1.53)	1.40 (1.14-1.76)	1.66 (1.30-2.15)	1.85 (1.42-2.44)	2.06 (1.52-2.77)	2.26 (1.60-3.14)	2.54 (1.73-3.63)	2.76 (1.82-4.00)
30-min	1.34 (1.10-1.68)	1.54 (1.26-1.92)	1.86 (1.52-2.33)	2.13 (1.73-2.68)	2.52 (1.98-3.27)	2.82 (2.16-3.72)	3.14 (2.32-4.23)	3.46 (2.44-4.80)	3.90 (2.65-5.55)	4.23 (2.80-6.13)
60-min	1.80 (1.47-2.24)	2.04 (1.67-2.54)	2.45 (2.00-3.06)	2.80 (2.27-3.52)	3.31 (2.60-4.30)	3.72 (2.85-4.90)	4.14 (3.06-5.59)	4.58 (3.24-6.36)	5.19 (3.53-7.41)	5.66 (3.74-8.20)
2-hr	2.24 (1.85-2.78)	2.54 (2.08-3.14)	3.03 (2.48-3.77)	3.47 (2.82-4.33)	4.10 (3.24-5.31)	4.61 (3.55-6.05)	5.14 (3.82-6.91)	5.70 (4.06-7.88)	6.48 (4.44-9.20)	7.10 (4.72-10.2)
3-hr	2.48 (2.05-3.06)	2.80 (2.31-3.46)	3.36 (2.76-4.16)	3.85 (3.15-4.79)	4.57 (3.63-5.91)	5.16 (3.99-6.75)	5.78 (4.32-7.75)	6.44 (4.61-8.87)	7.35 (5.06-10.4)	8.08 (5.40-11.6)
6-hr	2.84 (2.35-3.48)	3.26 (2.70-4.00)	3.99 (3.30-4.91)	4.64 (3.81-5.73)	5.60 (4.47-7.20)	6.38 (4.97-8.31)	7.21 (5.42-9.62)	8.09 (5.83-11.1)	9.32 (6.45-13.1)	10.3 (6.93-14.7)
12-hr	3.16 (2.63-3.84)	3.73 (3.11-4.55)	4.73 (3.92-5.78)	5.61 (4.63-6.88)	6.91 (5.54-8.84)	7.97 (6.23-10.3)	9.09 (6.87-12.1)	10.3 (7.45-14.0)	11.9 (8.31-16.7)	13.3 (8.97-18.8)
24-hr	3.63 (3.04-4.39)	4.26 (3.56-5.15)	5.39 (4.49-6.54)	6.41 (5.32-7.82)	7.96 (6.45-10.2)	9.26 (7.30-12.0)	10.6 (8.11-14.1)	12.2 (8.87-16.5)	14.3 (10.0-19.9)	16.0 (10.9-22.5)
2-day	4.37 (3.68-5.25)	4.90 (4.13-5.90)	5.94 (4.98-7.16)	6.93 (5.78-8.39)	8.51 (6.98-10.9)	9.90 (7.88-12.8)	11.4 (8.79-15.1)	13.1 (9.68-17.8)	15.6 (11.1-21.7)	17.7 (12.1-24.6)
3-day	4.83 (4.08-5.78)	5.39 (4.55-6.46)	6.46 (5.44-7.76)	7.48 (6.26-9.02)	9.09 (7.46-11.5)	10.5 (8.38-13.4)	12.0 (9.27-15.8)	13.7 (10.2-18.5)	16.2 (11.5-22.4)	18.2 (12.5-25.3)
4-day	5.20 (4.40-6.21)	5.80 (4.91-6.93)	6.92 (5.83-8.28)	7.96 (6.68-9.58)	9.58 (7.87-12.1)	11.0 (8.78-14.0)	12.5 (9.65-16.3)	14.2 (10.5-19.0)	16.6 (11.8-22.8)	18.5 (12.8-25.7)
7-day	6.18 (5.25-7.33)	6.82 (5.79-8.10)	7.98 (6.75-9.50)	9.04 (7.61-10.8)	10.7 (8.77-13.3)	12.0 (9.64-15.2)	13.5 (10.5-17.5)	15.1 (11.2-20.1)	17.4 (12.4-23.8)	19.2 (13.4-26.5)
10-day	7.04 (6.00-8.33)	7.74 (6.59-9.16)	8.97 (7.61-10.6)	10.1 (8.50-12.0)	11.7 (9.67-14.6)	13.1 (10.5-16.5)	14.6 (11.3-18.8)	16.2 (12.1-21.4)	18.5 (13.2-25.1)	20.3 (14.1-27.9)
20-day	9.53 (8.16-11.2)	10.5 (8.99-12.3)	12.1 (10.4-14.3)	13.6 (11.5-16.1)	15.6 (12.8-19.0)	17.2 (13.8-21.3)	18.8 (14.7-23.9)	20.5 (15.4-26.9)	22.9 (16.5-30.8)	24.7 (17.3-33.7)
30-day	11.7 (10.0-13.7)	12.9 (11.1-15.2)	15.0 (12.8-17.6)	16.7 (14.2-19.7)	19.0 (15.7-23.1)	20.9 (16.8-25.7)	22.7 (17.7-28.6)	24.5 (18.4-31.8)	27.0 (19.4-36.0)	28.8 (20.3-39.1)
45-day	14.5 (12.5-16.9)	16.2 (13.9-18.8)	18.7 (16.1-21.9)	20.8 (17.7-24.4)	23.6 (19.4-28.3)	25.6 (20.7-31.3)	27.6 (21.6-34.6)	29.6 (22.2-38.1)	32.1 (23.2-42.5)	33.9 (23.9-45.9)
60-day	17.1 (14.7-19.8)	19.0 (16.4-22.1)	22.0 (18.9-25.6)	24.3 (20.8-28.5)	27.4 (22.6-32.8)	29.7 (24.0-36.1)	31.8 (24.9-39.7)	33.8 (25.4-43.4)	36.3 (26.3-48.0)	38.1 (27.0-51.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

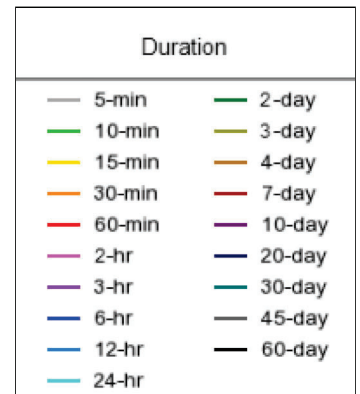
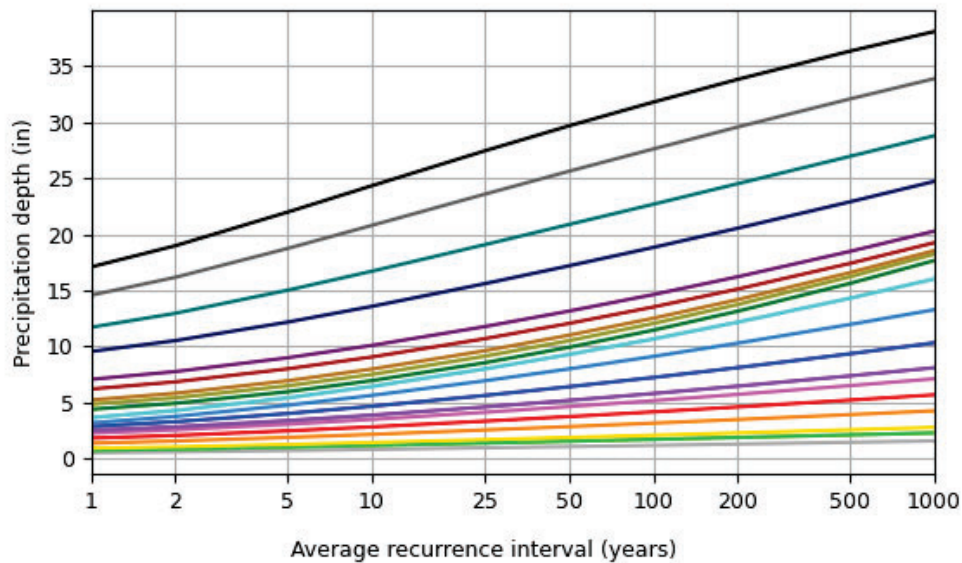
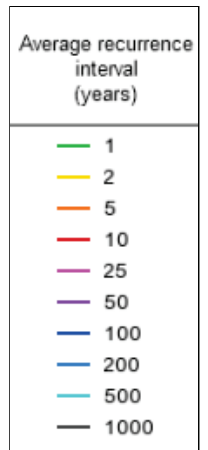
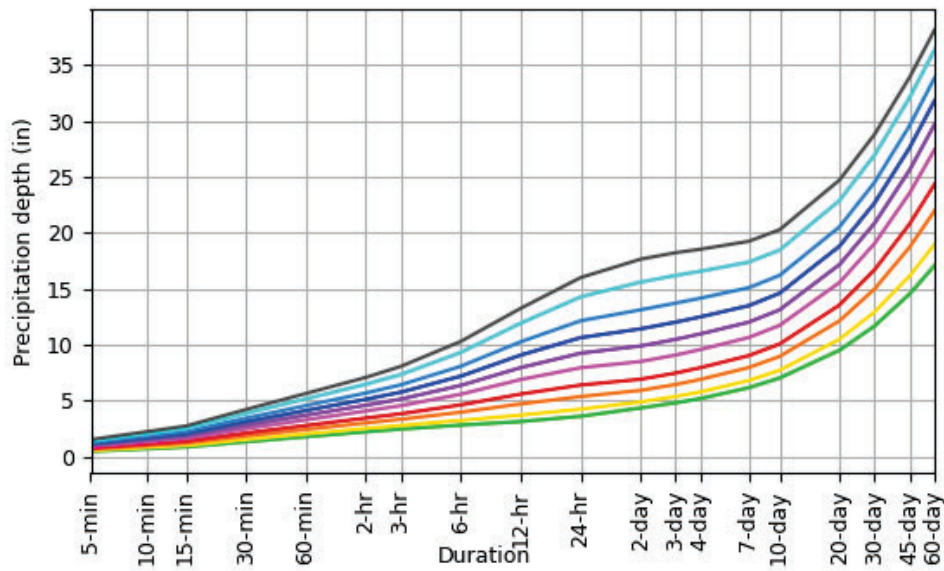
Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

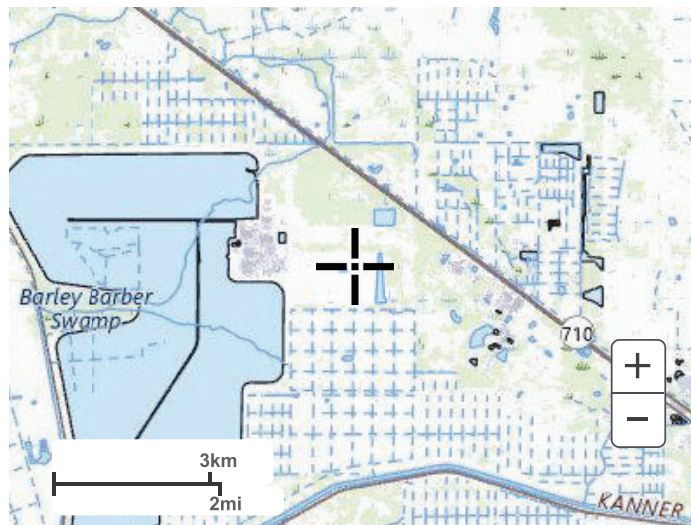
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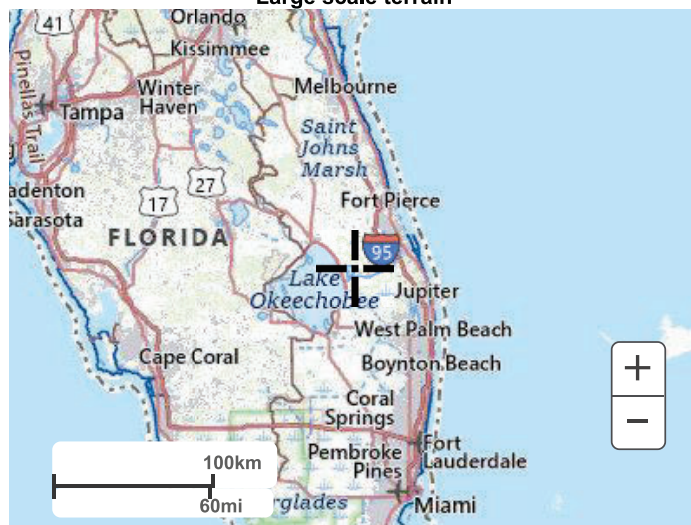
NOAA Atlas 14, Volume 9, Version 2

Created (GMT): Tue Jul 28 14:10:53 2026

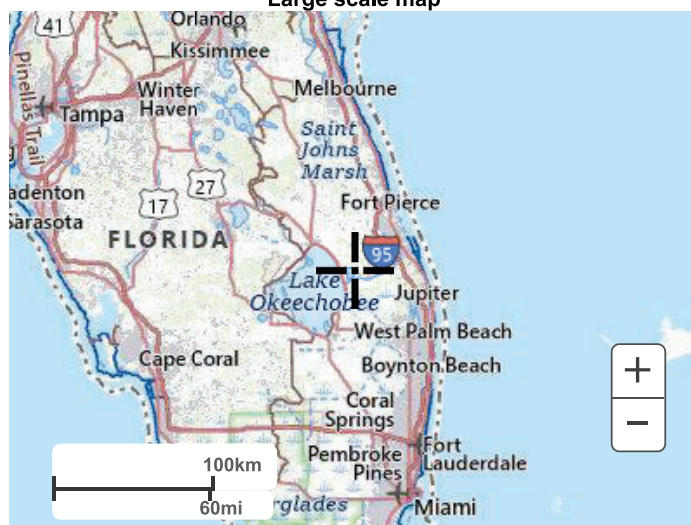
[Back to Top](#)**Maps & aerials****Small scale terrain**



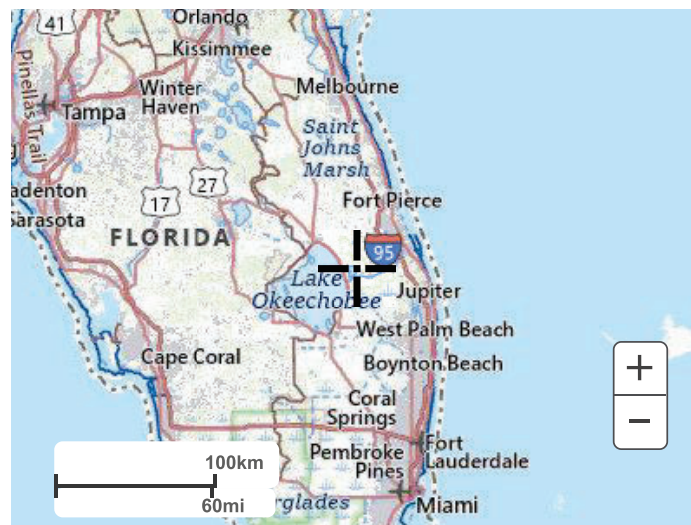
Large scale terrain



Large scale map



Large scale aerial

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[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

[Disclaimer](#)

APPENDIX C

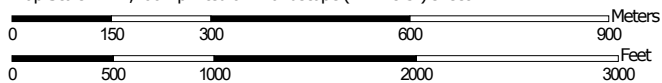
USDA NRCS SOIL MAP

Soil Map—Martin County, Florida



Soil Map may not be valid at this scale.

Map Scale: 1:11,400 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey

8/10/2026
Page 1 of 3

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

Water Features



Streams and Canals

Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Martin County, Florida

Survey Area Data: Version 24, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jan 18, 2022—Jan 30, 2022

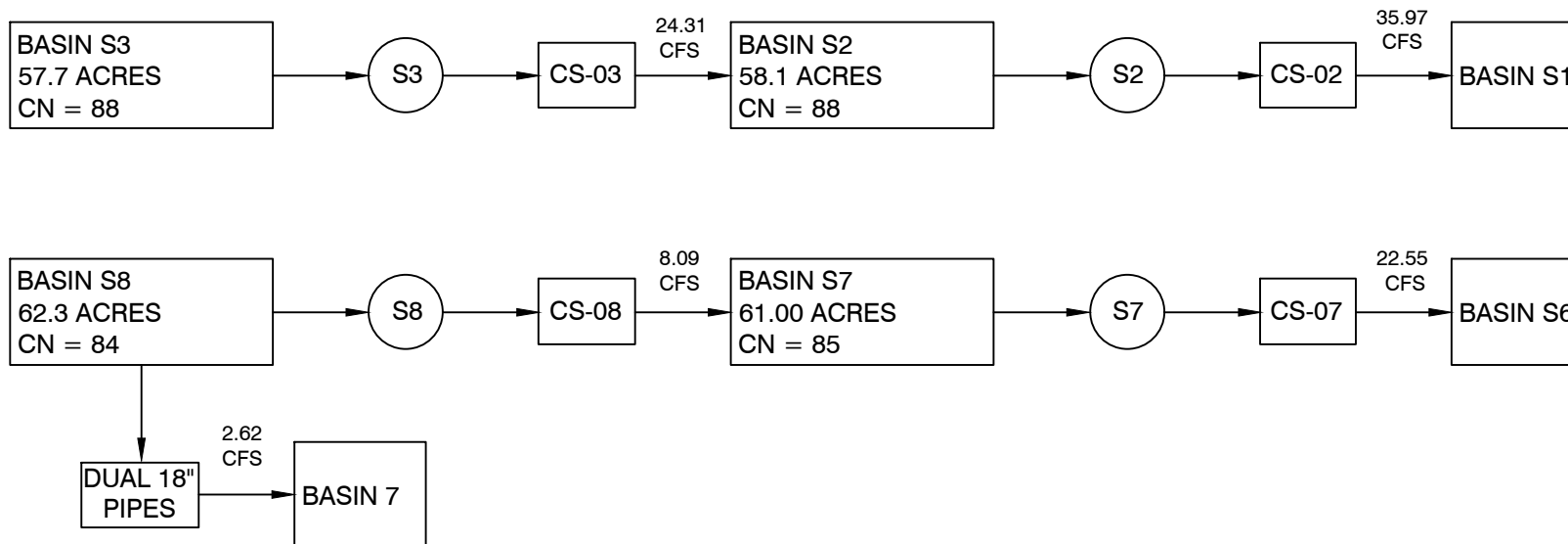
The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
2	Lawnwood and Myakka fine sands	50.6	8.4%
4	Waveland and Immokalee fine sands	265.9	43.9%
13	Placid and Basinger fine sands, depressional	0.4	0.1%
16	Oldsmar fine sand, 0 to 2 percent slopes	8.8	1.5%
17	Wabasso sand, 0 to 2 percent slopes	58.2	9.6%
19	Winder sand, frequently ponded, 0 to 1 percent slopes	7.3	1.2%
21	Pineda-Riviera fine sands association, 0 to 2 percent slopes	18.0	3.0%
36	Arents, 0 to 2 percent slopes	1.5	0.3%
38	Floridana fine sand, frequently ponded, 0 to 1 percent slopes	20.3	3.4%
47	Pinellas fine sand, 0 to 2 percent slopes	3.4	0.6%
49	Riviera fine sand, frequently ponded, 0 to 1 percent slopes	30.8	5.1%
56	Wabasso and Oldsmar fine sands, depressional	20.4	3.4%
57	Chobee muck, frequently ponded, 0 to 1 percent slopes	22.0	3.6%
58	Gator and Tequesta mucks	32.5	5.4%
66	Holopaw fine sand, 0 to 2 percent slopes	66.2	10.9%
Totals for Area of Interest		606.3	100.0%

APPENDIX D

ICPR MODEL



POST-DEVELOPMENT NODE-LINK DIAGRAM
FPL MARTIN 500kV LAYDOWN YARD

Simple Basin: S2

Scenario: Proposed
Node: S2
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 20.0000 min
Max Allowable Q: 9999.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 58.1000 ac
Curve Number: 88.0
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment:

Simple Basin: S3

Scenario: Proposed
Node: S3
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 20.0000 min
Max Allowable Q: 9999.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 57.7000 ac
Curve Number: 88.0
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment:

Simple Basin: S7

Scenario: Proposed
Node: S7
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number

Time of Concentration: 20.0000 min
Max Allowable Q: 9999.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 61.0000 ac
Curve Number: 85.0
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment:

Simple Basin: S8

Scenario: Proposed
Node: S8
Hydrograph Method: NRCS Unit Hydrograph
Infiltration Method: Curve Number
Time of Concentration: 20.0000 min
Max Allowable Q: 9999.00 cfs
Time Shift: 0.0000 hr
Unit Hydrograph: UH256
Peaking Factor: 256.0
Area: 62.3000 ac
Curve Number: 84.0
Ia/S: 0.00
% Impervious: 0.00
% DCIA: 0.00
% Direct: 0.00
Rainfall Name:

Comment:

Node: 7

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 27.00 ft
Warning Stage: 30.50 ft
Alert Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	27.00
0	0	0	9999.0000	27.00

Comment:

Node: S1

Scenario: Proposed
 Type: Time/Stage
 Base Flow: 0.00 cfs
 Initial Stage: 25.00 ft
 Warning Stage: 31.20 ft
 Alert Stage: 0.00 ft
 Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	25.00
0	0	0	9999.0000	25.00

Comment:

Node: S2

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 27.00 ft
 Warning Stage: 31.00 ft
 Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
27.00	5.8200	253519
28.00	6.9300	301871
29.00	8.0400	350222
30.00	13.4700	586753
31.00	44.3400	1931450

Comment:

Node: S3

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs

Initial Stage: 28.00 ft
 Warning Stage: 32.00 ft
 Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
28.00	5.8400	254390
29.00	6.9400	302306
30.00	8.0400	350222
31.00	13.4700	586753
32.00	44.4000	1934064

Comment:

Node: S6

Scenario: Proposed
 Type: Time/Stage
 Base Flow: 0.00 cfs
 Initial Stage: 27.00 ft
 Warning Stage: 33.20 ft
 Alert Stage: 0.00 ft
 Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	27.00
0	0	0	9999.0000	27.00

Comment:

Node: S7

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 29.00 ft
 Warning Stage: 32.50 ft
 Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
29.00	5.1500	224334
29.50	5.9800	260489
30.50	7.6700	334105
31.50	12.2300	532739
32.50	44.2400	1927094

Comment:

Node: S8

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 29.50 ft
 Warning Stage: 33.00 ft
 Alert Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
29.50	5.4000	235224
30.00	6.5300	284447
31.00	8.4800	369389
32.00	14.1000	614196
33.00	45.7300	1991999

Comment:

Pipe Link: Basin S8 Dual 18"

Scenario: Proposed
 From Node: S8
 To Node: 7
 Link Count: 2
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 80.00 ft
 FHWA Code: 0
 Entr Loss Coef: 0.00
 Exit Loss Coef: 0.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 31.50 ft
 Manning's N: 0.0240

Geometry: Circular

Max Depth: 1.50 ft

Bottom Clip

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

Top Clip

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

Downstream

Invert: 31.50 ft
 Manning's N: 0.0240

Geometry: Circular

Max Depth: 1.50 ft

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

Comment:

Drop Structure Link: CS-02

Scenario: Proposed
 From Node: S2
 To Node: S1
 Link Count: 1
 Pipe Flow Direction: Both
 Solution: Combine
 Increments: 0
 Pipe Count: 1
 Damping: 0.0000 ft
 Length: 30.00 ft

Upstream Pipe

Invert: 23.30 ft
 Manning's N: 0.0240

Geometry: Circular

Max Depth: 4.00 ft

Bottom Clip

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

Top Clip

Downstream Pipe

Invert: 23.30 ft
 Manning's N: 0.0240

Geometry: Circular

Max Depth: 4.00 ft

Default: 0.00 ft

Op Table:

Ref Node:

Manning's N: 0.0000

FHWA Code: 0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef: 0.50	Op Table:	Op Table:
Exit Loss Coef: 0.00	Ref Node:	Ref Node:
Bend Loss Coef: 0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location: 0.00 dec		
Energy Switch: Energy		

Pipe Comment:

Weir Component	
Weir: 1	Bottom Clip
Weir Count: 1	Default: 0.00 ft
Weir Flow Direction: Both	Op Table:
Damping: 0.0000 ft	Ref Node:
Weir Type: Sharp Crested Vertical	Top Clip
Geometry Type: Circular	Default: 0.00 ft
Invert: 25.85 ft	Op Table:
Control Elevation: 25.85 ft	Ref Node:
Max Depth: 0.70 ft	Discharge Coefficients
	Weir Default: 3.200
	Weir Table:
	Orifice Default: 0.600
	Orifice Table:

Weir Comment:

Weir Component	
Weir: 2	Bottom Clip
Weir Count: 1	Default: 0.00 ft
Weir Flow Direction: Both	Op Table:
Damping: 0.0000 ft	Ref Node:
Weir Type: Horizontal	Top Clip
Geometry Type: Rectangular	Default: 0.00 ft
Invert: 29.30 ft	Op Table:
Control Elevation: 26.00 ft	Ref Node:
Max Depth: 2.00 ft	Discharge Coefficients
Max Width: 5.00 ft	Weir Default: 3.200
Fillet: 0.00 ft	Weir Table:
	Orifice Default: 0.600
	Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-03		Upstream Pipe	Downstream Pipe
Scenario: Proposed		Invert: 25.90 ft	Invert: 25.90 ft
From Node: S3		Manning's N: 0.0240	Manning's N: 0.0240
To Node: S2		Geometry: Circular	Geometry: Circular
Link Count: 1		Max Depth: 3.00 ft	Max Depth: 3.00 ft

Pipe Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	30.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.50	Op Table:		Op Table:	
Exit Loss Coef:	0.00	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component		Bottom Clip	
Weir:	1	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Sharp Crested Vertical	Default:	0.00 ft
Geometry Type:	Circular	Op Table:	
Invert:	27.11 ft	Ref Node:	
Control Elevation:	27.11 ft	Discharge Coefficients	
Max Depth:	0.60 ft	Weir Default:	3.200
		Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component		Bottom Clip	
Weir:	2	Default:	0.00 ft
Weir Count:	1	Op Table:	
Weir Flow Direction:	Both	Ref Node:	
Damping:	0.0000 ft	Top Clip	
Weir Type:	Horizontal	Default:	0.00 ft
Geometry Type:	Rectangular	Op Table:	
Invert:	30.20 ft	Ref Node:	
Control Elevation:	27.00 ft	Discharge Coefficients	
Max Depth:	2.00 ft	Weir Default:	3.200
Max Width:	5.00 ft	Weir Table:	
Fillet:	0.00 ft	Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-07		Upstream Pipe	Downstream Pipe
Scenario:	Proposed	Invert: 26.90 ft	Invert: 26.90 ft
From Node:	S7	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	S6	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 3.00 ft	Max Depth: 3.00 ft
Pipe Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	30.00 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	0.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location:	0.00 dec		
Energy Switch:	Energy		

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Circular	Default:	0.00 ft
Invert:	28.07 ft	Op Table:	
Control Elevation:	28.07 ft	Ref Node:	
Max Depth:	0.70 ft	Discharge Coefficients	
		Weir Default:	3.200
		Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Weir Component			
Weir:	2	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	31.20 ft	Op Table:	
Control Elevation:	28.00 ft	Ref Node:	
Max Depth:	2.00 ft	Discharge Coefficients	
Max Width:	5.00 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: CS-08		Upstream Pipe	Downstream Pipe
Scenario:	Proposed	Invert: 27.20 ft	Invert: 27.20 ft
From Node:	S8	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	S7	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 3.00 ft	Max Depth: 3.00 ft
Pipe Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	30.00 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	0.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location:	0.00 dec		
Energy Switch:	Energy		

Pipe Comment:

Weir Component		
Weir:	1	Bottom Clip
Weir Count:	1	Default: 0.00 ft
Weir Flow Direction:	Both	Op Table:
Damping:	0.0000 ft	Ref Node:
Weir Type:	Sharp Crested Vertical	Top Clip
Geometry Type:	Circular	Default: 0.00 ft
Invert:	28.60 ft	Op Table:
Control Elevation:	28.60 ft	Ref Node:
Max Depth:	0.50 ft	Discharge Coefficients
		Weir Default: 3.200
		Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Weir Comment:

Weir Component		
Weir:	2	Bottom Clip
Weir Count:	1	Default: 0.00 ft
Weir Flow Direction:	Both	Op Table:
Damping:	0.0000 ft	Ref Node:
Weir Type:	Horizontal	Top Clip
Geometry Type:	Rectangular	Default: 0.00 ft
Invert:	32.00 ft	Op Table:
Control Elevation:	28.50 ft	Ref Node:

Max Depth: 2.00 ft
Max Width: 5.00 ft
Fillet: 0.00 ft

Discharge Coefficients

Weir Default: 3.200
Weir Table:
Orifice Default: 0.600
Orifice Table:

Weir Comment:

Drop Structure Comment:

Simulation: 100YR-72HR

Scenario: Proposed
Run Date/Time: 8/18/2026 9:31:21 AM
Program Version: StormWise 4.08.09

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	80.0000	0.1000
Max Calculation Time:		80.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:

Unit Hydrograph
Folder:

Curve Number Set:

Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain Global
Opt:

Rainfall Name: ~SFWMD-72
Rainfall Amount: 12.00 in
Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2
(1D):
Energy Switch (1D): Energy

Comment:

Simulation: 25YR-24HR

Scenario: Proposed
Run Date/Time: 8/18/2026 9:31:24 AM
Program Version: StormWise 4.08.09

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	30.0000
	Hydrology [sec]	Surface Hydraulics [sec]		
Min Calculation Time:	30.0000	0.1000		
Max Calculation Time:		30.0000		

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph
Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight: 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ftIA Recovery Time: 24.0000 hr
Ia/S: 0.20 dec
Smp/Man Basin Rain Opt: Global
Rainfall Name: ~SCSI-24
Rainfall Amount: 7.96 in
Storm Duration: 24.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area (1D): 100 ft2
Energy Switch (1D): Energy

Comment:

Simulation: 25YR-72HR

Scenario: Proposed
Run Date/Time: 8/18/2026 9:31:25 AM
Program Version: StormWise 4.08.09

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]
Min Calculation Time:	80.0000	0.1000
Max Calculation Time:		80.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:

Unit Hydrograph
Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:Green-Ampt Set:
Vertical Layers Set:
Impervious Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight: 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

IA Recovery Time: 24.0000 hr

Ia/S: 0.20 dec

Smp/Man Basin Rain: Global
Opt:Rainfall Name: ~SFWMD-72
Rainfall Amount: 9.09 in

Storm Duration: 72.0000 hr
Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2
(1D):
Energy Switch (1D): Energy

Comment:

Link Min/Max Conditions : Multi Item | (sim, name) [Proposed]

Sim Name	Link Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
100YR-72HR	Basin S8 Dual 18"	5.06	0.00	0.01	1.79	3.81	2.80
100YR-72HR	CS-02 - Pipe	56.21	0.00	1.66	0.00	0.00	0.00
100YR-72HR	CS-02 - Weir: 1	3.74	0.00	0.00	9.72	9.72	9.72
100YR-72HR	CS-02 - Weir: 2	52.52	0.00	0.07	5.25	5.25	5.25
100YR-72HR	CS-03 - Pipe	33.58	0.00	1.05	0.00	0.00	0.00
100YR-72HR	CS-03 - Weir: 1	1.29	0.00	0.00	4.56	4.56	4.56
100YR-72HR	CS-03 - Weir: 2	32.66	0.00	0.05	3.27	3.27	3.27
100YR-72HR	CS-07 - Pipe	42.92	0.00	1.41	0.00	0.00	0.00
100YR-72HR	CS-07 - Weir: 1	3.25	0.00	0.00	8.45	8.45	8.45
100YR-72HR	CS-07 - Weir: 2	40.37	0.00	0.06	3.09	3.09	3.09
100YR-72HR	CS-08 - Pipe	19.27	0.00	0.67	0.00	0.00	0.00
100YR-72HR	CS-08 - Weir: 1	0.78	0.00	0.00	3.96	3.96	3.96
100YR-72HR	CS-08 - Weir: 2	18.72	0.00	0.03	2.17	2.17	2.17
25YR-24HR	Basin S8 Dual 18"	2.15	0.00	0.01	1.26	2.98	2.12
25YR-24HR	CS-02 - Pipe	30.56	0.00	1.66	0.00	0.00	0.00
25YR-24HR	CS-02 - Weir: 1	3.62	0.00	0.00	9.40	9.40	9.40
25YR-24HR	CS-02 - Weir: 2	26.95	0.00	0.05	2.70	2.70	2.70
25YR-24HR	CS-03 - Pipe	19.63	0.00	1.05	0.00	0.00	0.00
25YR-24HR	CS-03 - Weir: 1	1.33	0.00	0.00	4.71	4.71	4.71
25YR-24HR	CS-03 - Weir: 2	18.53	0.00	0.03	2.38	2.38	2.38
25YR-24HR	CS-07 - Pipe	18.18	0.00	1.41	0.00	0.00	0.00
25YR-24HR	CS-07 - Weir: 1	3.25	0.00	0.00	8.45	8.45	8.45
25YR-24HR	CS-07 - Weir: 2	15.09	0.00	0.03	2.23	2.23	2.23
25YR-24HR	CS-08 - Pipe	5.63	0.00	0.67	0.00	0.00	0.00
25YR-24HR	CS-08 - Weir: 1	0.81	0.00	0.00	4.13	4.13	4.13
25YR-24HR	CS-08 - Weir: 2	4.93	0.00	0.02	1.53	1.53	1.53
25YR-72HR	Basin S8 Dual 18"	2.62	0.00	0.01	1.37	3.15	2.26

Sim Name	Link Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
25YR-72HR	CS-02 - Pipe	35.97	0.00	1.66	0.00	0.00	0.00
25YR-72HR	CS-02 - Weir: 1	3.66	0.00	0.00	9.51	9.51	9.51
25YR-72HR	CS-02 - Weir: 2	32.31	0.00	-0.06	2.87	2.87	2.87
25YR-72HR	CS-03 - Pipe	24.31	0.00	1.05	0.00	0.00	0.00
25YR-72HR	CS-03 - Weir: 1	1.27	0.00	0.00	4.50	4.50	4.50
25YR-72HR	CS-03 - Weir: 2	23.29	0.00	-0.05	2.54	2.54	2.54
25YR-72HR	CS-07 - Pipe	22.55	0.00	1.41	0.00	0.00	0.00
25YR-72HR	CS-07 - Weir: 1	3.25	0.00	0.00	8.45	8.45	8.45
25YR-72HR	CS-07 - Weir: 2	19.54	0.00	0.03	2.43	2.43	2.43
25YR-72HR	CS-08 - Pipe	8.09	0.00	0.67	0.00	0.00	0.00
25YR-72HR	CS-08 - Weir: 1	0.82	0.00	0.00	4.17	4.17	4.17
25YR-72HR	CS-08 - Weir: 2	7.42	0.00	0.02	1.76	1.76	1.76

Node Max Conditions : Multi Item | (sim, name) [Proposed]

Sim Name	Node Name	Warning Stage [ft]	Alert Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
100YR-72HR	7	30.50	0.00	27.00	0.0000	5.06	0.00	0
100YR-72HR	S1	31.20	0.00	25.00	0.0000	56.21	0.00	0
100YR-72HR	S2	31.00	0.00	30.49	0.0010	252.67	56.21	1247267
100YR-72HR	S3	32.00	0.00	31.35	-0.0010	229.69	33.58	1061147
100YR-72HR	S6	33.20	0.00	27.00	0.0000	42.92	0.00	0
100YR-72HR	S7	32.50	0.00	32.13	0.0010	242.36	42.92	1415395
100YR-72HR	S8	33.00	0.00	32.62	0.0010	242.96	24.33	1465305
25YR-24HR	7	30.50	0.00	27.00	0.0000	2.15	0.00	0
25YR-24HR	S1	31.20	0.00	25.00	0.0000	30.56	0.00	0
25YR-24HR	S2	31.00	0.00	30.01	0.0010	197.77	30.56	603919
25YR-24HR	S3	32.00	0.00	30.76	-0.0010	195.09	19.63	528814
25YR-24HR	S6	33.20	0.00	27.00	0.0000	18.18	0.00	0
25YR-24HR	S7	32.50	0.00	31.68	0.0010	198.60	18.18	789353
25YR-24HR	S8	33.00	0.00	32.23	0.0010	199.19	7.78	930684
25YR-72HR	7	30.50	0.00	27.00	0.0000	2.62	0.00	0
25YR-72HR	S1	31.20	0.00	25.00	0.0000	35.97	0.00	0
25YR-72HR	S2	31.00	0.00	30.10	-0.0010	173.63	35.97	727081
25YR-72HR	S3	32.00	0.00	30.86	-0.0010	171.18	24.31	553188
25YR-72HR	S6	33.20	0.00	27.00	0.0000	22.55	0.00	0
25YR-72HR	S7	32.50	0.00	31.78	0.0010	177.52	22.55	916358
25YR-72HR	S8	33.00	0.00	32.30	0.0010	179.06	10.72	1029890