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VP Buildings

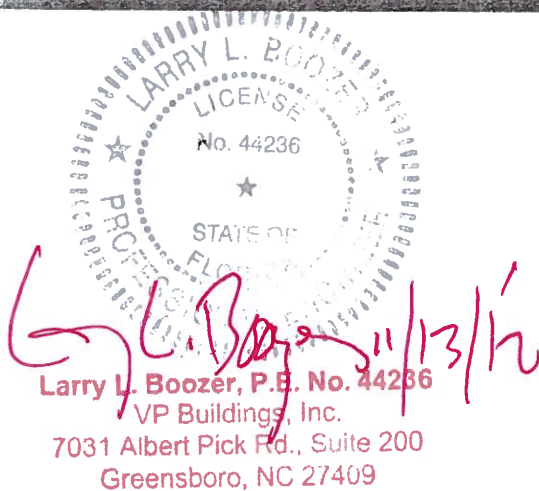
3200 Players Club Circle
Memphis, TN 38125-8843

STRUCTURAL DESIGN DATA

Project: FPL-Dry Storage Building
Name: 2706-12158-FPL-Dry Storage Building
Builder PO #: 2706-12158
Jobsite: 9760 SW 344th St

City, State: Homestead, Florida 33035
County: Miami-Dade
Country: United States

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Larry L. Boozer, P.E. No. 44236
VP Buildings, Inc.
7031 Albert Pick Rd., Suite 200
Greensboro, NC 27409



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Letter of Certification

Contact: Noel Evans
Name: Lemartec Eng. & Const. Corp.
Address: 11740 SW 80th Street
Third Floor
City, State: Miami, Florida 33183
Country: United States

Project: FPL-Dry Storage Building
Builder PO #: 2706-12158
Jobsite: 9760 SW 344th St
City, State: Homestead, Florida 33035
County, Country: Miami-Dade, United States

This is to certify that the above referenced project has been designed in accordance with the applicable portions of the Building Code specified below.
All loading and building design criteria shown below have been specified by contract and applied in accordance with the building code.

Overall Building Description

Shape	Overall Width	Overall Length	Floor Area (sq. ft.)	Wall Area (sq. ft.)	Roof Area (sq. ft.)	Max. Eave Height	Min. Eave Height 2	Max. Roof Pitch	Min. Roof Pitch	Peak Height
Dry Stge Building	50/0/0	70/0/0	3500	3756	3527	15/0/0	15/0/0	1.500:12	1.500:12	18/1/8

Loads and Codes - Shape: Dry Stge Building

City: Homestead County: Miami-Dade
Building Code: 2010 Florida State Building Code (HVHZ=SFI)
Building Use: Essential Facility

State: Florida
Built Up: 10AISC - ASD
Cold Form: 04AISI - ASD

Country: United States
Rainfall: 12.00 inches per hour
3000.00 psi Concrete

Dead and Collateral Loads

Collateral Gravity: 25.00 psf
Collateral Uplift: 0.00 psf

Roof Covering + Second. Dead Load: 2.94 psf
Frame Weight (assumed for seismic): 0.00 psf

Roof Live Load

Roof Live Load: 20.00 psf Not Reducible

Wind Load

Wind Speed: 186.00 mph (Vult.)
144.00 mph (Vasd.)

Snow Load
N/A

Seismic Load
N/A

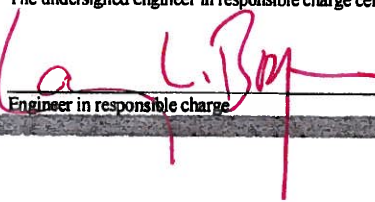
The 'Low Rise' Method is Used
Wind Exposure (Factor): C (0.849)
Parts Wind Exposure Factor: 0.849
Wind Enclosure: Enclosed
Wind Importance Factor: 1.000
Topographic Factor: 1.0000
Hurricane Prone Region
Windborne Debris Region
Impact Resistant Covering
Base Elevation: 0/0/0
Primary Zone Strip Width: 10/0/0
Parts / Portions Zone Strip Width: 5/0/0
Basic Wind Pressure: 63.90 psf

Building design loads and governing building code is provided by the Builder and is not validated by Varco Pruden Buildings, a division of BlueScope Buildings North America, Inc. The Builder is responsible for contacting the local Building Official or project Design Professional to obtain all code and loading information for this specific building site.

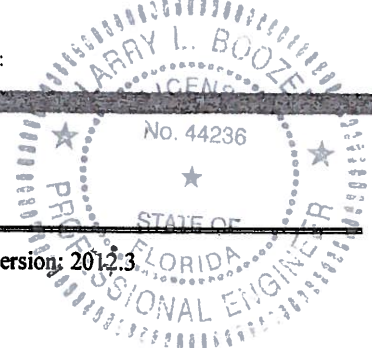
The design of this building is in accordance with Varco Pruden Buildings, a division of BlueScope Buildings North America, Inc. design practices which have been established based upon pertinent procedures and recommendations of the Standards listed in the Building Code or later editions.

This certification DOES NOT apply to the design of the foundation or other on-site structures or components not supplied by Varco Pruden Buildings, a division of BlueScope Buildings North America, Inc., nor does it apply to unauthorized modifications to building components. Furthermore, it is understood that certification is based upon the premise that all components will be erected or constructed in strict compliance with pertinent documents for this project. Varco Pruden Buildings, a division of BlueScope Buildings North America, Inc. DOES NOT provide general review of erection during or after building construction unless specifically agreed to in the contract documents.

The undersigned engineer in responsible charge certifies that this building has been designed in accordance with the contract documents as indicated in this letter.


Engineer in responsible charge

Date: 11/13/12 Engineer's Seal:



File: 2706-12158-FPL-Dry Storage Building

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Version: 2012.3



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Reaction Summary Report**Shape: Dry Stge Building**

Builder Contact: James Partridge
Name: Lemartec Eng. & Const. Corp.
Address: 11740 SW 80th Street
Third Floor
City, State Zip: Miami, Florida 33183
Country: United States

Project: FPL-Dry Storage Building
Builder PO #: 2706-12158
Jobsite: 9760 SW 344th St

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County, Country: Miami-Dade, United States

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Frame Weight (assumed for seismic): 0.00 psf

Roof Live Load

Roof Live Load: 20.00 psf Not Reducible

Wind Load

Wind Speed: 186.00 mph (Vult.)
144.00 mph (Vasd.)
The 'Low Rise' Method is Used
Wind Exposure (Factor): C (0.849)
Parts Wind Exposure Factor: 0.849
Wind Enclosure: Enclosed
Wind Importance Factor: 1.000
Topographic Factor: 1.0000
Hurricane Prone Region
Windborne Debris Region
Impact Resistant Covering
Base Elevation: 0/0/0
Primary Zone Strip Width: 10/0/0
Parts / Portions Zone Strip Width: 5/0/0
Basic Wind Pressure: 63.90 psf

Snow Load
N/A

Seismic Load
N/A

Load Type Descriptions

D Material Dead Weight
CG Collateral Load for Gravity Cases
L Roof Live Load
^ASL Alternate Span Live Load, Shifted Left
L> Live - Notional Right
S Snow Load
*US1 Unbalanced Snow Load 1, Shifted Left
*US2 Unbalanced Snow Load 2, Shifted Left
SS Sliding Snow Load
PF1 Partial Load, Full, 1 Span
PF2 Partial Load, Full, 2 Spans
S> Snow - Notional Right
SMS Specified Min. Roof Snow
<SMS Specified Min. Roof Snow - Notional Left
PF1 Partial Load, Half Span 2
W1> Wind Load, Case 1, Right
W2> Wind Load, Case 2, Right
W3> Wind Load, Case 3, Right
W4> Wind Load, Case 4, Right
W5> Wind Load, Case 5, Right
W6> Wind Load, Case 6, Right
WP Wind Load, Parallel to Ridge
WPL Wind Load, || Ridge, Left
WPA2 Wind Parallel - Ref A, Case 2
WPB2 Wind Parallel - Ref B, Case 2
WPC2 Wind Parallel - Ref C, Case 2
WPD2 Wind Parallel - Ref D, Case 2
<WB1 Wind Brace Reaction, Case 1, Left
<WB2 Wind Brace Reaction, Case 2, Left
<WB3 Wind Brace Reaction, Case 3, Left

C Collateral Load
CU Collateral Load for Wind Cases
ASL^ Alternate Span Live Load, Shifted Right
PL2 Partial Live, Full, 2 Spans
<L Live - Notional Left
US1* Unbalanced Snow Load 1, Shifted Right
US2* Unbalanced Snow Load 2, Shifted Right
SD Snow Drift Load
RS Rain Surcharge Load
PH1 Partial Load, Half, 1 Span
PH2 Partial Load, Half, 2 Spans
<S Snow - Notional Left
SMS> Specified Min. Roof Snow - Notional Right
PF1 Partial Load, Half Span 1
W Wind Load
<W1 Wind Load, Case 1, Left
<W2 Wind Load, Case 2, Left
<W3 Wind Load, Case 3, Left
<W4 Wind Load, Case 4, Left
<W5 Wind Load, Case 5, Left
<W6 Wind Load, Case 6, Left
WPR Wind Load, || Ridge, Right
WPA1 Wind Parallel - Ref A, Case 1
WPB1 Wind Parallel - Ref B, Case 1
WPC1 Wind Parallel - Ref C, Case 1
WPD1 Wind Parallel - Ref D, Case 1
WB1> Wind Brace Reaction, Case 1, Right
WB2> Wind Brace Reaction, Case 2, Right
WB3> Wind Brace Reaction, Case 3, Right
WB4> Wind Brace Reaction, Case 4, Right



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<WB4	Wind Brace Reaction, Case 4, Left	WB5>	Wind Brace Reaction, Case 5, Right
<WB5	Wind Brace Reaction, Case 5, Left	WB6>	Wind Brace Reaction, Case 6, Right
<WB6	Wind Brace Reaction, Case 6, Left	MW	Minimum Wind Load
MWB	Minimum Wind Bracing Reaction	E	Seismic Load
E>	Seismic Load, Right	<E	Seismic Load, Left
EG	Vertical Seismic Effect	EG+	Vertical Seismic Effect, Additive
EG-	Vertical Seismic Effect, Subtractive	EB>	Seismic Brace Reaction, Right
<EB	Seismic Brace Reaction, Left	FL	Floor Live Load
FL*	Alternate Span Floor Live Load, Shifted Right	*FL	Alternate Span Floor Live Load, Shifted Left
FD	Floor Dead Load	AL	Auxiliary Live Load
AL*>	Auxiliary Live Load, Right, Right	*AL>	Auxiliary Live Load, Right, Left
<AL*	Auxiliary Live Load, Left, Right	<*AL	Auxiliary Live Load, Left, Left
AL*	Aux Live, Right	*AL	Aux Live, Left
AL*>(1)	Auxiliary Live Load, Right, Right, Aisle 1	*AL>(1)	Auxiliary Live Load, Right, Left, Aisle 1
<AL*(1)	Auxiliary Live Load, Left, Right, Aisle 1	<*AL(1)	Auxiliary Live Load, Left, Left, Aisle 1
AL*(1)	Aux Live, Right, Aisle 1	*AL(1)	Aux Live, Left, Aisle 1
AL*>(2)	Auxiliary Live Load, Right, Right, Aisle 2	*AL>(2)	Auxiliary Live Load, Right, Left, Aisle 2
<AL*(2)	Auxiliary Live Load, Left, Right, Aisle 2	<*AL(2)	Auxiliary Live Load, Left, Left, Aisle 2
AL*(2)	Aux Live, Right, Aisle 2	*AL(2)	Aux Live, Left, Aisle 2
AL*>(3)	Auxiliary Live Load, Right, Right, Aisle 3	*AL>(3)	Auxiliary Live Load, Right, Left, Aisle 3
<AL*(3)	Auxiliary Live Load, Left, Right, Aisle 3	<*AL(3)	Auxiliary Live Load, Left, Left, Aisle 3
AL*(3)	Aux Live, Right, Aisle 3	*AL(3)	Aux Live, Left, Aisle 3
AL*>(4)	Auxiliary Live Load, Right, Right, Aisle 4	*AL>(4)	Auxiliary Live Load, Right, Left, Aisle 4
<AL*(4)	Auxiliary Live Load, Left, Right, Aisle 4	<*AL(4)	Auxiliary Live Load, Left, Left, Aisle 4
AL*(4)	Aux Live, Right, Aisle 4	*AL(4)	Aux Live, Left, Aisle 4
AL*>(5)	Auxiliary Live Load, Right, Right, Aisle 5	*AL>(5)	Auxiliary Live Load, Right, Left, Aisle 5
<AL*(5)	Auxiliary Live Load, Left, Right, Aisle 5	<*AL(5)	Auxiliary Live Load, Left, Left, Aisle 5
AL*(5)	Aux Live, Right, Aisle 5	*AL(5)	Aux Live, Left, Aisle 5
ALB	Aux Live Bracing Reaction	ALB>	Aux Live Bracing Reaction, Right
<ALB	Aux Live Bracing Reaction, Left	WALB>	Wind, Aux Live Bracing Reaction, Right
<WALB	Wind, Aux Live Bracing Reaction, Left	ALB>(1)	Aux Live Bracing Reaction, Right, Aisle 1
<ALB(1)	Aux Live Bracing Reaction, Left, Aisle 1	WALB>(1)	Wind, Aux Live Bracing Reaction, Right, Aisle 1
<WALB(1)	Wind, Aux Live Bracing Reaction, Left, Aisle 1	ALB>(2)	Aux Live Bracing Reaction, Right, Aisle 2
<ALB(2)	Aux Live Bracing Reaction, Left, Aisle 2	WALB>(2)	Wind, Aux Live Bracing Reaction, Right, Aisle 2
<WALB(2)	Wind, Aux Live Bracing Reaction, Left, Aisle 2	ALB>(3)	Aux Live Bracing Reaction, Right, Aisle 3
<ALB(3)	Aux Live Bracing Reaction, Left, Aisle 3	WALB>(3)	Wind, Aux Live Bracing Reaction, Right, Aisle 3
<WALB(3)	Wind, Aux Live Bracing Reaction, Left, Aisle 3	ALB>(4)	Aux Live Bracing Reaction, Right, Aisle 4
<ALB(4)	Aux Live Bracing Reaction, Left, Aisle 4	WALB>(4)	Wind, Aux Live Bracing Reaction, Right, Aisle 4
<WALB(4)	Wind, Aux Live Bracing Reaction, Left, Aisle 4	ALB>(5)	Aux Live Bracing Reaction, Right, Aisle 5
<ALB(5)	Aux Live Bracing Reaction, Left, Aisle 5	WALB>(5)	Wind, Aux Live Bracing Reaction, Right, Aisle 5
<WALB(5)	Wind, Aux Live Bracing Reaction, Left, Aisle 5	WALB	Wind, Aux Live Bracing Reaction
AD	Auxiliary Dead Load	U0	User Defined Load
U1	User Defined Load - 1	U2	User Defined Load - 2
U3	User Defined Load - 3	U4	User Defined Load - 4
U5	User Defined Load - 5	U6	User Defined Load - 6
U7	User Defined Load - 7	U8	User Defined Load - 8
U9	User Defined Load - 9	UB	User Brace Reaction
UB1	User Brace Reaction - 1	UB2	User Brace Reaction - 2
UB3	User Brace Reaction - 3	UB4	User Brace Reaction - 4
UB5	User Brace Reaction - 5	UB6	User Brace Reaction - 6
UB7	User Brace Reaction - 7	UB8	User Brace Reaction - 8
UB9	User Brace Reaction - 9	R	Rain Load
T	Temperature Load	V	Shear



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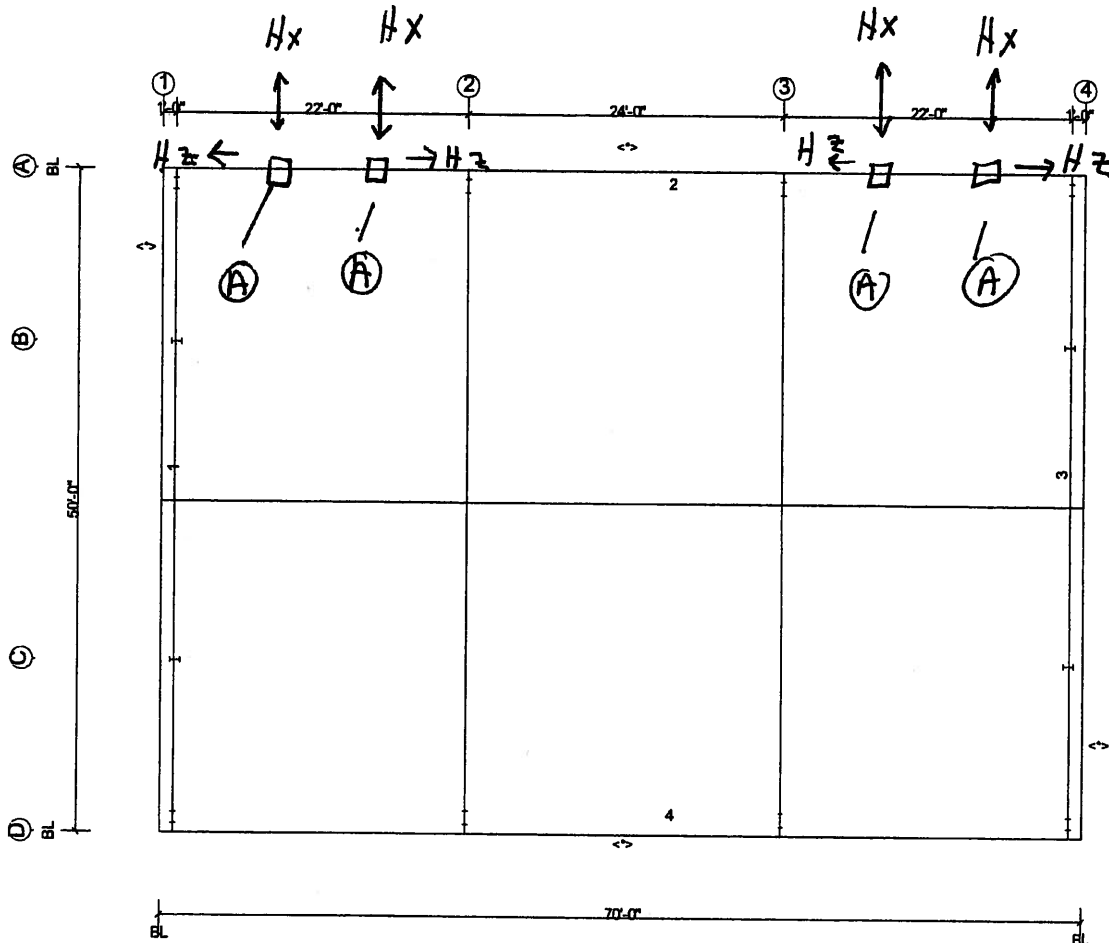
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Overall Building Description

Shape	Overall Width	Overall Length	Floor Area (sq. ft.)	Wall Area (sq. ft.)	Roof Area (sq. ft.)	Max. Eave Height	Min. Eave Height 2	Max. Roof Pitch	Min. Roof Pitch	Peak Height
Dry Stge Building	50/0/0	70/0/0	3500	3756	3527	15/0/0	15/0/0	1.500:12	1.500:12	18/1/8

Overall Shape Description

Roof 1	Roof 2	From Grid	To Grid	Width	Length	Eave Ht.	Eave Ht. 2	Pitch	Pitch 2	Dist. to Ridge	Peak Height
A	B	1-A	1-D	50/0/0	70/0/0	15/0/0	15/0/0	1.500:12	1.500:12	25/0/0	18/1/8



<*> The building is designed with bracing diagonals in the designated bays. Column base reactions, base plates and anchor rods are affected by this bracing and diagonals may not be relocated without consulting the building supplier's engineer.

(A) T.S. 8x8x1/4 w/ (2) 1" ϕ A.R.'s

$$\left. \begin{array}{l} Hx = \pm 4.2^K \\ Hz = 11.25^K \end{array} \right\} V_{ULT}$$



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Wall: 4, Frame at: 1/0/0

Design Load Combinations - Framing

No.	Origin	Factor	Application	Description
1	System	1.000	1.0 D + 1.0 CG + 1.0 L>	D + CG + L>
2	System	1.000	1.0 D + 1.0 CG + 1.0 <L	D + CG + <L
3	System	1.000	1.0 D + 1.0 CG + 0.600 W1>	D + CG + W1>
4	System	1.000	1.0 D + 1.0 CG + 0.600 <W1	D + CG + <W1
5	System	1.000	1.0 D + 1.0 CG + 0.600 W2>	D + CG + W2>
6	System	1.000	1.0 D + 1.0 CG + 0.600 <W2	D + CG + <W2
7	System	1.000	0.600 MW	MW - Wall: 1
8	System	1.000	0.600 MW	MW - Wall: 2
9	System	1.000	0.600 MW	MW - Wall: 3
10	System	1.000	0.600 MW	MW - Wall: 4
11	System	1.000	0.600 D + 0.600 CU + 0.600 W1>	D + CU + W1>
12	System	1.000	0.600 D + 0.600 CU + 0.600 <W1	D + CU + <W1
13	System	1.000	0.600 D + 0.600 CU + 0.600 W2>	D + CU + W2>
14	System	1.000	0.600 D + 0.600 CU + 0.600 <W2	D + CU + <W2
15	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPR + 0.600 WB1>	D + CG + WPR + WB1>
16	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPR + 0.600 WB1>	D + CU + WPR + WB1>
17	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPR + 0.600 <WB1	D + CG + WPR + <WB1
18	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPR + 0.600 <WB1	D + CU + WPR + <WB1
19	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPR + 0.600 WB2>	D + CG + WPR + WB2>
20	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPR + 0.600 WB2>	D + CU + WPR + WB2>
21	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPR + 0.600 <WB2	D + CG + WPR + <WB2
22	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPR + 0.600 <WB2	D + CU + WPR + <WB2
23	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPL + 0.600 WB3>	D + CG + WPL + WB3>
24	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPL + 0.600 WB3>	D + CU + WPL + WB3>
25	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPL + 0.600 <WB3	D + CG + WPL + <WB3
26	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPL + 0.600 <WB3	D + CU + WPL + <WB3
27	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPL + 0.600 WB4>	D + CG + WPL + WB4>
28	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPL + 0.600 WB4>	D + CU + WPL + WB4>
29	System Derived	1.000	1.0 D + 1.0 CG + 0.600 WPL + 0.600 <WB4	D + CG + WPL + <WB4
30	System Derived	1.000	0.600 D + 0.600 CU + 0.600 WPL + 0.600 <WB4	D + CU + WPL + <WB4
31	System Derived	1.000	0.600 MWB	MWB - Wall: 1
32	System Derived	1.000	0.600 MWB	MWB - Wall: 2
33	System Derived	1.000	0.600 MWB	MWB - Wall: 3
34	System Derived	1.000	0.600 MWB	MWB - Wall: 4



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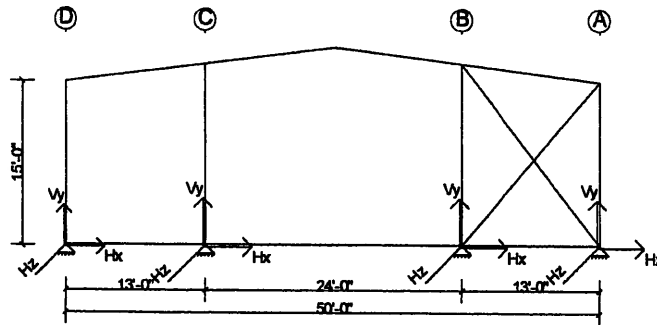
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Wall: 4, Frame at: 1/0/0
Frame ID: Post & Beam

Frame Type: Post & Beam



Values shown are resisting forces of the foundation.

Base Connection Design is Based on 3000.00 (psi) Concrete

Reactions - Unfactored Load Type at Frame Cross Section: 1

Type X-Loc Grid 1 - Grid 2 Base Plate W x L (in.) Base Plate Thickness (in.) Anchor Rod Qty/Diam. (in.) Column Base Elev.		Exterior Column 0/0/0 1-D 8 X 11 0.750 4 - 0.750 100'-0"		Interior Column 13/0/0 1-C 8 X 10 0.750 4 - 0.750 100'-0"		Interior Column 37/0/0 1-B 8 X 10 0.750 4 - 0.750 100'-0"		Exterior Column 50/0/0 1-A 8 X 11 0.750 4 - 0.750 100'-0"		
Load Type	Desc.	Hx	Vy	Hx	Hx	Hx	Hx	Hx	Vy	
D	Frm	-	0.3	-	-	1.2	-	-	1.2	-
CG	Frm	-	1.1	-	-	6.4	-	-	6.4	-
L>	Frm	-	1.3	-	-	7.7	-	-	7.7	-
<L	Frm	-	1.3	-	-	7.7	-	-	7.7	-
W1>	Frm	-2.9	-3.1	-	8.6	-17.5	-	8.6	-13.8	-4.0
<W1	Frm	4.0	-2.6	-	-7.7	-13.8	-	-7.7	-17.5	2.9
W2>	Frm	-4.8	-1.7	-	-	-12.0	-	-	-8.3	-2.1
<W2	Frm	2.1	-1.2	-	-	-8.3	-	-	-12.0	4.8
MW	Frm	-	-	-	-	-	-	-	-	-
MW	Frm	-	0.0	-	-	-0.2	-	-	-0.1	1.3
MW	Frm	-	-	-	-	-	-	-	-	-
MW	Frm	-1.3	0.3	-	-	-0.1	-	-	-0.2	0.0
CU	Frm	-	-	-	-	-	-	-	-	-
WPR	Frm	3.5	-4.2	-	-	-16.7	-	-	-11.2	-3.5
WB1>	Brc	-	-	-	-	0.0	-5.1	-	-6.2	6.1
<WB1	Brc	-	-	-	-	-	-	-	6.5	4.9
WB2>	Brc	-	-	-	-	0.0	-5.1	-	-6.2	6.2
<WB2	Brc	-	-	-	-	-	-	-	6.5	4.9
WPL	Frm	3.5	-2.2	-	-	-11.2	-	-	-16.7	-3.5
WB3>	Brc	-	-	-	-	0.0	-5.1	-	-6.2	6.2
<WB3	Brc	-	-	-	-	-	-	-	6.6	4.9
WB4>	Brc	-	-	-	-	0.0	-5.1	-	-6.2	6.2
<WB4	Brc	-	-	-	-	-	-	-	6.6	4.9
MWB	Brc	-	-	-	-	-	-	-	2.1	1.6
MWB	Brc	-	-	-	-	-	-	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-
MWB	Brc	-	-	-	-	-1.7	-	-	-2.0	2.0
L	Frm	-	1.3	-	-	7.7	-	-	7.7	1.3

Maximum Combined Reactions Summary with Factored Loads - Framing



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Note: All reactions based on 2nd order structural analysis using the Direct Analysis Method

X-Loc	Grid	Hz left (-Hx) (k)	Load Case	Hz Right (Hx) (k)	Load Case	Hz In (-Hz) (k)	Load Case	Hz Out (Hz) (k)	Load Case	Uplift (-Vy) (k)	Load Case	Vrt Down (Vy) (k)	Load Case	Mom cw (-Mzz) (in-k)	Load Case	Mom ccw (Mzz) (in-k)	Load Case
0/0/0	I-D	2.9	5	2.4	4	-	-	-	-	2.4	16	2.8	1	-	-	-	-
13/0/0	I-C	-	-	-	-	4.6	4	5.1	3	9.8	11	15.2	1	-	-	-	-
37/0/0	I-B	3.1	15	-	-	4.6	4	5.1	3	13.0	28	15.2	1	-	-	-	-
50/0/0	I-A	2.4	3	2.9	6	-	-	-	-	6.3	26	3.8	19	-	-	-	-

Bracing

X-Loc	Grid	Description
37/0/0	I-B	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.



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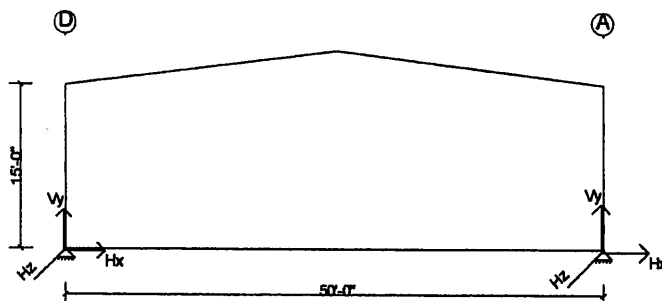
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Wall: 4, Frame at: 23/0/0

Frame ID: Rigid Frame

Frame Type: Rigid Frame



Values shown are resisting forces of the foundation.

Base Connection Design is Based on 3000.00 (psi) Concrete

Reactions - Unfactored Load Type at Frame Cross Section: 2

Type X-Loc Grid1 - Grid2 Base Plate W x L (in.) Base Plate Thickness (in.) Anchor Rod Qty/Diam. (in.) Column Base Elev.		Exterior Column 0/0/0 2-D 8 X 13 0.750 4 - 0.750 100'-0"			Exterior Column 50/0/0 2-A 8 X 13 0.750 4 - 0.750 100'-0"					
Load Type	Desc.	Hx	Hx	Vy	Hx	Hx	Vy			
D	Frm	1.1	-	2.5	-1.1	-	2.5	-	-	-
CG	Frm	7.3	-	14.4	-7.3	-	14.4	-	-	-
L>	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-
<L	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-
W1>	Frm	-23.9	-	-34.4	2.9	-	-23.8	-	-	-
<W1	Frm	-2.9	-	-23.8	23.9	-	-34.4	-	-	-
W2>	Frm	-21.1	-	-21.1	0.1	-	-10.5	-	-	-
<W2	Frm	-0.1	-	-10.5	21.1	-	-21.1	-	-	-
MW	Frm	-	-	-	-	-	-	-	-	-
MW	Frm	1.6	-	1.0	4.5	-	-1.0	-	-	-
MW	Frm	-	-	-	-	-	-	-	-	-
MW	Frm	-4.5	-	-1.0	-1.6	-	1.0	-	-	-
CU	Frm	-	-	-	-	-	-	-	-	-
WPR	Frm	-6.0	-	-29.7	7.5	-	-24.2	-	-	-
WB1>	Brc	0.2	-9.9	-6.2	-0.2	-9.2	-5.8	-	-	-
<WB1	Brc	-0.2	-	6.2	0.2	-	5.7	-	-	-
WB2>	Brc	0.2	-9.9	-6.2	-0.2	-9.2	-5.8	-	-	-
<WB2	Brc	-0.2	-	6.3	0.2	-	5.7	-	-	-
WPL	Frm	-7.5	-	-24.2	6.0	-	-29.7	-	-	-
WB3>	Brc	0.2	-9.2	-5.8	-0.2	-9.9	-6.3	-	-	-
<WB3	Brc	-0.2	-	5.9	0.2	-	6.3	-	-	-
WB4>	Brc	0.2	-9.2	-5.8	-0.2	-9.9	-6.4	-	-	-
<WB4	Brc	-0.2	-	5.8	0.2	-	6.3	-	-	-
MWB	Brc	0.1	-3.3	-2.1	-0.1	-3.3	-2.1	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-
MWB	Brc	-0.1	-	2.1	0.1	-	2.1	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-
L	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-

Maximum Combined Reactions Summary with Factored Loads - Framing



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Note: All reactions based on 2nd order structural analysis using the Direct Analysis Method

X-Loc	Grid	Hz left (-Hx) (k)	Load Case	Hz Right (Hx) (k)	Load Case	Hz In (-Hz) (k)	Load Case	Hz Out (Hz) (k)	Load Case	Uplift (-Vy) (k)	Load Case	Vrt Down (Vy) (k)	Load Case	Mom cw (-Mzz) (in-k)	Load Case	Mom ccw (Mzz) (in-k)	Load Case
0/0/0	2-D	13.6	11	17.2	1	5.9	19	-	-	20.0	20	34.2	1	-	-	-	-
50/0/0	2-A	17.2	1	13.6	12	5.9	27	-	-	20.1	28	34.2	1	-	-	-	-

Bracing

X-Loc	Grid	Description
0/0/0	2-D	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.
50/0/0	2-A	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.



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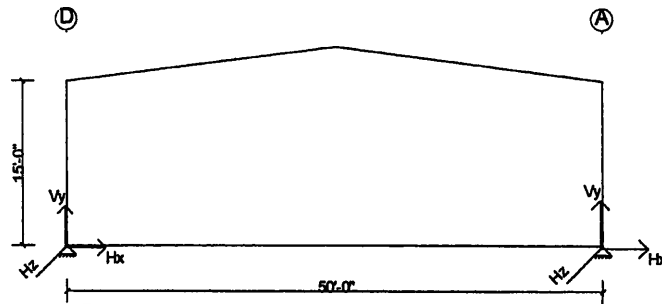
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Wall: 4, Frame at: 47/0/0
Frame ID: Rigid Frame

Frame Type: Rigid Frame



Values shown are resisting forces of the foundation.

Base Connection Design is Based on 3000.00 (psi) Concrete

Reactions - Unfactored Load Type at Frame Cross Section: 3

Type		Exterior Column			Exterior Column						
X-Loc		0/0/0			50/0/0						
Grid1 - Grid2		3-D			3-A						
Base Plate W x L (in.)		8 X 13			8 X 13						
Base Plate Thickness (in.)		0.750			0.750						
Anchor Rod Qty/Diam. (in.)		4 - 0.750			4 - 0.750						
Column Base Elev.		100'-0"			100'-0"						
Load Type	Desc.	Hx	Hz	Vy	Hx	Hz	Vy				
D	Frm	1.1	-	2.5	-1.1	-	2.5	-	-	-	-
CG	Frm	7.3	-	14.4	-7.3	-	14.4	-	-	-	-
L>	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-	-
<L	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-	-
W1>	Frm	-23.9	-	-34.4	2.9	-	-23.8	-	-	-	-
<W1	Frm	-2.9	-	-23.8	23.9	-	-34.4	-	-	-	-
W2>	Frm	-21.1	-	-21.1	0.1	-	-10.5	-	-	-	-
<W2	Frm	-0.1	-	-10.5	21.1	-	-21.1	-	-	-	-
MW	Frm	-	-	-	-	-	-	-	-	-	-
MW	Frm	1.6	-	1.0	4.5	-	-1.0	-	-	-	-
MW	Frm	-	-	-	-	-	-	-	-	-	-
MW	Frm	-4.5	-	-1.0	-1.6	-	1.0	-	-	-	-
CU	Frm	-	-	-	-	-	-	-	-	-	-
WPR	Frm	-6.0	-	-29.7	7.5	-	-24.2	-	-	-	-
WB1>	Brc	-0.2	-	6.2	0.2	-	5.7	-	-	-	-
<WB1	Brc	0.2	9.9	-6.2	-0.2	9.2	-5.7	-	-	-	-
WB2>	Brc	-0.2	-	6.2	0.2	-	5.8	-	-	-	-
<WB2	Brc	0.2	9.9	-6.3	-0.2	9.2	-5.7	-	-	-	-
WPL	Frm	-7.5	-	-24.2	6.0	-	-29.7	-	-	-	-
WB3>	Brc	-0.2	-	5.9	0.2	-	6.3	-	-	-	-
<WB3	Brc	0.2	9.2	-5.9	-0.2	9.9	-6.3	-	-	-	-
WB4>	Brc	-0.2	-	5.8	0.2	-	6.3	-	-	-	-
<WB4	Brc	0.2	9.2	-5.9	-0.2	9.9	-6.3	-	-	-	-
MWB	Brc	-0.1	-	2.1	0.1	-	2.1	-	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-	-
MWB	Brc	0.1	3.3	-2.1	-0.1	3.3	-2.1	-	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-	-
L	Frm	8.8	-	17.2	-8.8	-	17.2	-	-	-	-

Maximum Combined Reactions Summary with Factored Loads - Framing



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Note: All reactions based on 2nd order structural analysis using the Direct Analysis Method

X-Loc	Grid	Hz left (-Hx) (k)	Load Case	Hz Right (Hx) (k)	Load Case	Hz In (-Hz) (k)	Load Case	Hz Out (Hz) (k)	Load Case	Uplift (-Vy) (k)	Load Case	Vrt Down (Vy) (k)	Load Case	Mom cw (-Mzz) (in-k)	Load Case	Mom ccw (Mzz) (in-k)	Load Case
0/0/0	3-D	13.6	11	17.2	1	-	-	5.9	21	20.1	22	34.2	1	-	-	-	-
50/0/0	3-A	17.2	1	13.6	12	-	-	5.9	29	20.1	30	34.2	1	-	-	-	-

Bracing

X-Loc	Grid	Description
0/0/0	3-D	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.
50/0/0	3-A	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.



12-23530 PERMIT DLR

Date: 10/30/2012

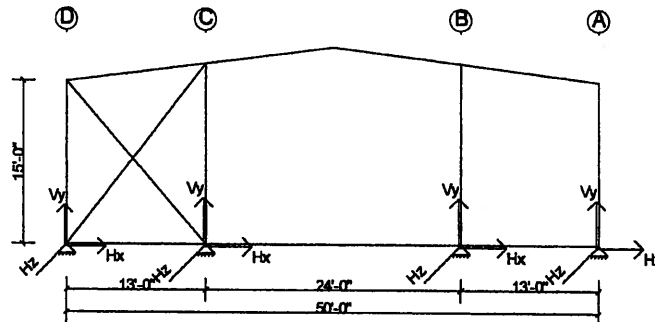
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Wall: 4, Frame at: 69/0/0

Frame ID: Post & Beam

Frame Type: Post & Beam



Values shown are resisting forces of the foundation.

Base Connection Design is Based on 3000.00 (psi) Concrete

Reactions - Unfactored Load Type at Frame Cross Section: 4

Type X-Loc Grid1 - Grid2 Base Plate W x L (in.) Base Plate Thickness (in.) Anchor Rod Qty/Diam. (in.) Column Base Elev.		Exterior Column 0/0/0 4-D 8 X 11 0.750 4 - 0.750 100'-0"		Interior Column 13/0/0 4-C 8 X 10 0.750 4 - 0.750 100'-0"			Interior Column 37/0/0 4-B 8 X 10 0.750 4 - 0.750 100'-0"			Exterior Column 50/0/0 4-A 8 X 11 0.750 4 - 0.750 100'-0"		
Load Type	Desc.	Hx	Vy	Hx	Hx	Vy	Hx	Hx	Vy	Hx	Vy	
D	Frm	-	0.3	-	-	1.2	-	-	1.2	-	0.3	-
CG	Frm	-	1.1	-	-	6.4	-	-	6.4	-	1.1	-
L>	Frm	-	1.3	-	-	7.7	-	-	7.7	-	1.3	-
<L	Frm	-	1.3	-	-	7.7	-	-	7.7	-	1.3	-
W1>	Frm	-2.9	-3.1	-	8.6	-17.5	-	8.6	-13.8	-4.0	-2.6	-
<W1	Frm	4.0	-2.6	-	-7.7	-13.8	-	-7.7	-17.5	2.9	-3.1	-
W2>	Frm	-4.8	-1.7	-	-	-12.0	-	-	-8.3	-2.1	-1.2	-
<W2	Frm	2.1	-1.2	-	-	-8.3	-	-	-12.0	4.8	-1.7	-
MW	Frm	-	-	-	-	-	-	-	-	-	-	-
MW	Frm	-	0.0	-	-	-0.2	-	-	-0.1	1.3	0.3	-
MW	Frm	-	-	-	-	-	-	-	-	-	-	-
MW	Frm	-1.3	0.3	-	-	-0.1	-	-	-0.2	-	0.0	-
CU	Frm	-	-	-	-	-	-	-	-	-	-	-
WPR	Frm	3.5	-4.2	-	-	-16.7	-	-	-11.2	-3.5	-2.2	-
WB1>	Brc	5.3	-6.5	-	-	6.5	-	-	0.0	-	-	-
<WB1	Brc	-	6.1	-5.1	-	-6.2	-	-	-	-	-	-
WB2>	Brc	5.3	-6.5	-	-	6.5	-	-	0.0	-	-	-
<WB2	Brc	-	6.2	-5.1	-	-6.2	-	-	-	-	-	-
WPL	Frm	3.5	-2.2	-	-	-11.2	-	-	-16.7	-3.5	-4.2	-
WB3>	Brc	5.3	-6.6	-	-	6.6	-	-	-	-	-	-
<WB3	Brc	-	6.2	-5.1	-	-6.2	-	-	0.0	-	-	-
WB4>	Brc	5.3	-6.6	-	-	6.6	-	-	-	-	-	-
<WB4	Brc	-	6.2	-5.1	-	-6.2	-	-	0.0	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-	-	-
MWB	Brc	-	2.0	-1.7	-	-2.0	-	-	-	-	-	-
MWB	Brc	-	-	-	-	-	-	-	-	-	-	-
MWB	Brc	1.7	-2.1	-	-	2.1	-	-	-	-	-	-
L	Frm	-	1.3	-	-	7.7	-	-	7.7	-	1.3	-

Maximum Combined Reactions Summary with Factored Loads - Framing



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Note: All reactions based on 2nd order structural analysis using the Direct Analysis Method

X-Loc	Grid	Hz left (-Hx) (k)	Load Case	Hz Right (Hx) (k)	Load Case	Hz In (-Hz) (k)	Load Case	Hz Out (Hz) (k)	Load Case	Uplift (-Vy) (k)	Load Case	Vrt Down (Vy) (k)	Load Case	Mom cw (-Mzz) (in-k)	Load Case	Mom ccw (Mzz) (in-k)	Load Case
0/0/0	4-D	2.9	5	5.3	23	-	-	-	-	6.3	20	3.8	29	-	-	-	-
13/0/0	4-C	3.1	17	-	-	4.6	4	5.1	3	13.0	22	15.2	1	-	-	-	-
37/0/0	4-B	-	-	-	-	4.6	4	5.1	3	9.8	12	15.2	1	-	-	-	-
50/0/0	4-A	2.4	3	2.9	6	-	-	-	-	2.4	24	2.8	1	-	-	-	-

Bracing

X-Loc	Grid	Description
13/0/0	4-C	Diagonal bracing at base is attached to column. Reactions ARE included with frame reactions.